

# Investigation of mechanical and durability properties of recycled aggregate concrete containing crumb rubber considering a new model of elastic modulus

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## 1 Abstract

2 Natural gravel and Sand are growing more and more expensive due to their scarcity. Therefore, replacing  
3 natural aggregates with recycled materials has been a concern of researchers. In this paper, crumb rubber  
4 was utilized to supersede a few of the aggregate in percentages of 5, 10, and 15 aggregate volumes. In  
5 concrete containing 15% rubber crumbs, 10% silica fume, and 10% zeolite were used along with crumb  
6 rubber. The findings of this investigation indicated that concrete containing 15% crumb rubber causes the  
7 greatest decline in compressive strength. Comparative to control concrete, compressive strength was  
8 decreased by 35% at 28 days and 36% at 7 days by substituting 15% of crumb rubber with aggregate  
9 volume. Additionally, it was discovered that the compressive strength of concrete containing 15% rubber  
10 crumbs raised by 23% and 33% at 28 days, and 21% and 34% at 7 days, respectively, when the mixture  
11 contained 10% zeolite and 10% silica fume. Also, the greatest decline in flexural strength, tensile strength,  
12 and modulus of elasticity was related to concrete containing 15% crumb rubber, which was improved by  
13 adding 10% pozzolan. Finally, an elastic modulus prediction model for this type of concrete (recycled  
14 aggregate concrete) is presented.

15 **Keywords:** Crumb rubber; Elastic modulus model; Silica fume; Zeolite; Mechanical properties; Concrete  
16 durability; Recycled aggregate concrete

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17 **1. Introduction**

18 A tire is made up of elastomeric compositions that contain steel fiber cord [1]. The disposal of old crumb  
19 rubber has been a severe environmental issue worldwide [2, 3]. Each year, millions of tires approach the  
20 end of their serviceable lives, resulting in a significant number of non-biodegradable solid waste in the  
21 environment [4]. There are several ways to get rid of scrap tires, including burning [5] and landfilling [6]  
22 or as mulch on sports fields and asphalt binder modifiers [7]. Stockpiled tires generate health,  
23 environmental, and economic difficulties due to pollution of the air, water, and soil [6, 8–10]. Tire  
24 burning, which was the most convenient and inexpensive approach to disposal, now creates significant  
25 fire dangers [9, 11–13].

26 Recognizing this issue, government institutions, business stakeholders, and the scientific community have  
27 banded together to find scientific solutions to recycle all types of waste materials [14]. Various waste  
28 materials are now being explored for recycling options [15, 16].

29 The utilization of discarded rubber in technology to create concrete, the commonly utilized material in  
30 buildings, and uses a considerable quantity of natural resources, has been one of the study paths in the  
31 latest years. It is a resource-saving and environmentally responsible way to utilize discarded crumb rubber  
32 as aggregates in cement concrete.

33 Numerous experiments have already been conducted to see if rubber could be utilized as a substitution for  
34 aggregates. The density of concrete is influenced by using crumb rubber as aggregates. With go up in the  
35 proportion of rubber crumbs in concrete, a diminish in terms of a weight unit of rubberized concrete has  
36 been observed [17–20]. According to several experimental research, this decline in the unit weight of  
37 concrete was caused by the low specific gravity of the rubber utilized [19, 21–24]. The potential of crumb  
38 rubber grains to trap air in their uneven surface pattern was blamed for the fall in density [25].  
39 Nagrockiene and Girskas [26] discovered that raising the amount of zeolite in concrete boosts its density.

40 The workability of concrete with rubber crumb fibers declines when the percentage of crumb rubber  
41 fibers increases [27–30]. According to reports, zeolite can reduce the workability and setting time of  
42 traditional concrete [31]. Oikonomou and Mauridou [23] demonstrated that the workability of rubber  
43 reduces by up to 15% as the amount of rubber increases.

44 The compressive strength slowly declines, as the percentage of rubber particles in the concrete grows [2,  
45 20, 32–35]. Coarse rubber crumbs and cement paste have a poorer connection than fine rubber crumbs  
46 and cement paste, according to Topcu [36], which impacts compressive strength. Sohrabi and Karbalaie  
47 [37], and Guneyisi et al. [13], at water to cement ratio (0.5), concrete's compression strength adding  
48 crumb rubber and silica fume was examined. If the substitution of crumb rubber does not account for  
49 more than 20% of the overall aggregate content, significant decreases in compressive strength could be  
50 prevented [30,38]. It was discovered that adding silica fume to the mix increased compressive strength.  
51 The rationale was that nanometric gaps in cement paste were filled, resulting in a denser structure.  
52 Tammana [35], showed that compressive strength decreased by 38%, at 28 days after increasing the  
53 aggregate replacement with rubber by 20%.

54 Adding scrap rubber tires in rubberized concrete affects its flexural strength. According to the literature,  
55 adding more crumb rubber increases flexural strength [22, 39–45]. The size of the waste rubber used  
56 determines this inconsistency in behavior [21, 32, 46, and 47]. Crumb rubber (CR) in concrete has low  
57 stiffness [48] and good suppleness [49] and is composed of particles of various sizes [50], cleanliness  
58 [51], contents [52], shapes [53], and CR surface finish quality [54] all influence the attributes of CR  
59 concrete (CRC).

60 When fine rubber aggregates were utilized instead of coarse rubber aggregates, Thiruppathi [55] reported  
61 a more significant decrease in the static elasticity modulus. According to Benazzouk et al. [56],  
62 rubberized concrete has a decrease in dynamic modulus of elasticity than control concrete. The ultrasonic  
63 wave absorption by concrete was attributed to the decline in the dynamic elastic modulus. According to

64 several studies, elasticity modulus declines as the quantity of rubber components increases [30, 57–60].  
65 Also, machine learning techniques such as artificial neural networks have recently been employed in the  
66 context of structural engineering like prediction of structural behavior [61, 62] or material properties [63]  
67 including modules of elasticity, durability, and compression strength.

68 The most critical aspect in determining concrete durability is permeability. Ganjian et al. [29] carried out  
69 a test investigation to explore the influence of scrap crumb rubber on water permeability when using a  
70 constant W/C ratio (0.5). Water permeability was enhanced by substituting coarse aggregate with chipped  
71 rubber aggregates. The reduced connection between particles in the concrete mixture was attributed to the  
72 increased water permeability.

73 All studies used waste tire rubber of different sizes, but there was no exact information about residual  
74 rubber on each sieve. Therefore, in this study, 4.75, 9.5, 12.5, and 14.75 were employed, and the ratio of  
75 water to cement was continual. The effect of substituting 10% of the cement with silica fume and zeolite  
76 and substituting 5%, 10%, and 15% of the coarse aggregate with coarse crumb rubber was also examined.  
77 The present study evaluated mechanical properties, including compressive strength, flexural strength,  
78 tensile strength, modulus of elasticity, and durability, including electrical resistance, ultrasonic, porosity,  
79 and water absorption.

## 80 **2. Experimental program**

### 81 2.1. Material properties

82 Type two Portland cement, silica fume, and zeolite were consumed in this research. Table 1 exhibits the  
83 physical and chemical properties of cement, silica fume, and zeolite. This study divided coarse aggregate,  
84 fine aggregate, and sand for preparing concrete mixes. The maximum sizes of them were 19, 9.5, and 4.75  
85 (mm), respectively. Coarse gravel, fine gravel, and sand with Density 2.69, 2.67, and 2.6, and water  
86 absorption of 2.63, 2.38, and 4.61 were used, respectively. An Iranian national guideline was used for

87 mixing aggregates [64]. The selection of aggregate consumption range in this study has been made  
88 according to the national method of the Iranian concrete mixing plans [64]. This study used natural coarse  
89 aggregate, fine aggregate, and sand with a particle size distribution and a 19 mm maximal particle size to  
90 prepare concrete mixtures. Based on the grain-size distribution, the distribution curve is a combination of  
91 the results between curves A19 and B19.

**Table 1.** Cement's physical and chemical characteristics and pozzolans.

92 In this paper, the remaining rubber on the sieves was 4.75, 9.5, 12.5, and 14.75. The distribution curve  
93 with a density of 1.12 was used, and the ratio of water to cement was continual. Coarse waste rubber  
94 chips replaced percentages of 5, 10, and 15 of the coarse aggregate, and the influence of replacing 10% of  
95 cement with silica fume with a density of 2.27 and zeolite with a density of 2.7 with 15% of waste rubber  
96 was investigated. This article provides details on the mixing ratios in Table 2.

**Table 2.** Proportions of crude rubber and pozzolan in concrete containing crude rubber and pozzolan.

### 97 **3. Experimental program**

#### 98 3.1. Testing of stone materials

99 Density tests were performed in a saturated state with a dry surface and water adsorption percentage of  
100 sand according to ASTM C128-88 [65]. Density tests were performed in the saturated state with dry  
101 surface and water adsorption percentage of coarse aggregates and fine aggregate according to ASTM  
102 C127-88 [66]. The standard test was performed to determine the water content of the aggregate by drying  
103 it according to ASTM C5666-89 [67]. This experiment was performed to calculate the laboratory

104 humidity of aggregate before each sample was made to correct the amount of mixed water. Grain-size  
105 aggregate distribution by the standard was done according to ASTM C136-84a [68].

### 106 3.2. Test for slump

107 ASTM C143 [69] was used to conduct the slump test.

### 108 3.3. Unit weight of fresh concrete

109 The density of fresh concrete was established in accordance with ASTM C138 [70].

### 110 3.4. Test of compressive strength

111 In this study, in line with BS 1881-116: 1983 [71], compressive strength tests were performed on a square  
112 standard material of 10 \*10 \*10 cm on days 7 and 28.

### 113 3.5. Tensile strength test

114 The Brazilian technique or halving in accordance with ASTM C496-90 was used in the experiment to  
115 estimate the tensile strength of models that are 20 cm in height and have a 10 cm diameter [72].

### 116 3.6. Testing of the flexural strength

117 Testing of the flexural strength of concrete was performed using a simple beam method and loading in the  
118 middle point according to ASTM C293-07 [73]. For this purpose, concrete samples with dimensions of 40  
119 \* 10 \* 10 cm were made.

120

121 3.7. Testing the elastic modulus

122

123 Testing the elastic modulus of samples that are 20 cm in height and have a 10 cm diameter by the pressure  
124 method was performed according to BS EN 1992-1-1 [74].

125

126 3.8. Ultrasonic test

127 Ultrasonic testing of cubic samples with dimensions of 10 \* 10 \* 10 cm was performed according to  
128 ASTM C597 [75].

129 3.9. Electrical resistance test

130 Electrical resistance test of cubic samples with dimensions of 10 \* 10 \* 10 cm was performed according  
131 to ASTM C1760 [76].

132 3.10. Water absorption and porosity testing of concrete

133 This experiment was carried out in accordance with ASTM C642 [77]. The results of 3 cubic tests of  
134 concrete were averaged to determine the percentage by weight of water absorption at the age of 28 days.

135 **4. Results and discussions**

136 4.1. Workability (fresh concrete)

137 Table 3 indicates the difference in density and slump of the specimens compared with the concrete  
138 without shredded rubber, silica fume, and zeolite. The slump gradually reduces, as the percentage of  
139 shredded rubber in concrete rises. Slump declines and prevents the consistency of concrete due to the  
140 constant quantity of water used in the combination and the increase in the quantity of crumb rubber. The  
141 decreased inter-particle friction between the rubber and other components could be the cause of the  
142 reaction [30]. When crumb rubber was mixed with concrete, the density of the material decreased due to  
143 the particular gravity of shredded rubber becoming less than that of natural aggregates [32, 49]. This  
144 reduction was slightly improved when pozzolan was replaced by cement in the rubber concrete mix.

**Table 3.** Slump test results and specific weight.

145 4.2. Compressive strength

146 Figure 1 shows the trend of changes in compressive strength. The samples were examined at seven and 28  
147 days of age. It has been noticed that when the quantity of rubber crumbs raised, the compressive strength  
148 of the concrete containing rubber reduced. Compressive strength declined by 6%, 18%, and 35% at 28  
149 days after increasing the aggregate replacement with rubber crumbs by 5 %, 10%, and 15%, respectively,  
150 comparison to the control mix samples. There was also a decline at the age of seven days. Compressive  
151 strength is reduced because of the formation of porosity, which may have arisen due to rubber particles.  
152 Cracks from around the crumb rubber in concrete containing crumb rubber while loading and these  
153 materials might speed up the failure of the matrix of cement and crumb rubber. Grains of crumb rubber  
154 might be considered porosity in the rubber concrete mix, increasing porosity and lowering the strength  
155 [78]. Figure 2 shows that the compressive strength was improved by adding silica fume and zeolite to the  
156 rubber concrete mix. Replacing 15% rubber with aggregate containing 10% silica fume and zeolite



157 revealed that the compressive strength rose by 33% and 23%, respectively, at 28 days compared to  
158 CR15%. This decrease was also seen at the age of seven days. This increase is due to the filling properties  
159 of silica fume and zeolite by refined grains and also creating good adhesive between rubber crumbs and  
160 the cement matrix.

Figure 1. Compressive strength of crumb rubber concrete at 7 and 28 days.

Figure 2. Compressive strength of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

#### 161 4.3. Flexural strength

162 Seven and 28 days of flexural strength of crumb rubber concrete are indicated in Figure 3. It has been  
163 discovered that as the quantity of crumb rubber in the concrete grows, the flexural strength of the concrete  
164 falls. A reduction of 6%, 13%, and 27% in flexural strength was observed when 5%, 10%, and 15%  
165 rubber aggregate was used in place of coarse aggregate, respectively, at the age of 28 days compared to  
166 the control mix samples. This decrease was also seen at the age of seven days. The decreased flexural  
167 strength is a weak connection between the cement paste and the rubber pieces [28]. By replacing 15%  
168 rubber with aggregate containing 10% silica fume and zeolite, it was observed that the flexural strength  
169 rose by 10% and 11%, respectively, on days 28 and 7, rose by 21% and 13%, respectively contrasted to  
170 CR15%, it shows figure 4.

Figure 3. Flexural strength of crumb rubber concrete at 7 and 28 days.

Figure 4. Flexural strength of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

171 4.4. Tensile strength

172 Figure 5 shows the results of 7 and 28 days of tensile strength testing with and without crumb particles  
173 rubber at various amounts. As seen in the graph, the tensile strength declines as the percentage of waste  
174 rubber replaced develops. The tensile strength is lowered by around 27%, 14%, and 21% in samples  
175 containing 5%, 10%, and 15% rubber crumbs at 28 days, respectively, contrasted to the control mix  
176 samples. This decrease was also seen at the age of seven days. The decrease is because as the crack  
177 expands, the pressure separates the surface between the rubber particles and the cement paste. Therefore,  
178 the rubber acts as a hole, leading to rapid concrete deterioration. By enhancing the interfacial transition  
179 zone (ITZ), pozzolans including silica fume and zeolite can strengthen the bond between the rubber and  
180 the binder, preventing strength loss [79, 80]. The findings indicate, 10% SF and 10% ZE improved the  
181 tensile strength of concrete including 15% crumb rubber at the age of 28- days by 13% and 9%,  
182 respectively, and at the age of 7-day by 12% and 2%, respectively compared to CR15%. It is shown in  
183 figure 6.

184 The results indicated a special relationship between compressive and tensile strength, figure 7. As the  
185 compressive strength decreased, the tensile strength decreased, and vice versa. In CR15%, the  
186 compressive strength was reduced by 35% and the tensile strength by 21%. Lower reduction of tensile  
187 strength than compressive strength can be assumed that rubber as a soft material can act as a barrier  
188 against the growth of cracks in concrete. Also, in CR15%SF10% compressive and tensile strength rose by  
189 33% and 13% contrasted to CR15%.

Figure 5. Tensile strength of crumb rubber concrete at 7 and 28 days.

Figure 6. Tensile strength of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

Figure 7. Relationship among 28-day compressive strength and 28 days tensile strength of concrete containing shredded rubber, silica fume, and zeolite.

190 4.5. Modulus of elasticity

191 As illustrated in Figure 8, the rise of rubber particles in concrete affects the modulus of elasticity of the  
192 concrete mixtures. A reduction of 5%, 8%, and 15% at 28-day was noticed in the elastic modulus  
193 aggregate comparison to the control mix samples when 5%, 10%, and 15% coarse aggregate and fine  
194 aggregate were substituted with rubber aggregate, respectively. This decline can be explained to the fact  
195 that the small holes in the rubber granules, which are usually made with water, cause cavities in the  
196 mixture; as the volume of the rubber increases, the porosity increases, and the bend and curvature  
197 increase, resulting in a decrease in stiffness and modulus of elasticity. Pozzolans can be used to  
198 compensate for this decrease. Because pozzolans have filler properties and fill porosity, the elastic  
199 modulus is improved. As shown in figure 9, the modulus of concrete elasticity improvement is 5%, 6%  
200 for CR15%SF10% and CR15%ZE10% at 28-day compared to CR15%, respectively. The results of the  
201 researchers [29, 81] also confirm this.

202 Figure 10 shows the relationship between the modulus of elasticity and porosity of 28-day concrete  
203 containing rubber particles, silica fume, and zeolite from this study. The modulus of elasticity decreases  
204 with increasing porosity. The results showed that CR15% had more porosity and less modulus of  
205 elasticity than the other samples; adding 10% pozzolan to this mixture improved porosity, and modulus of  
206 elasticity was observed.

Figure 8. Modulus of elasticity of crumb rubber concrete at 7 and 28 days.

Figure 9. Modulus of elasticity of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

Figure 10. Relation between modulus of elasticity and 28 days porosity of concrete containing crumb rubber, silica fume, zeolite.

207 The results of this study were made a comparison to the international codes in Table 4 to understand the  
208 research method better and review the data gathered. Codes had a reduced forecast than the research's  
209 outcomes, as seen in Table 4. The NBR 6118 code has the best forecast for the modulus of elasticity  
210 among the proposed codes. Other codes cannot accurately predict the elastic modulus of rubber concrete  
211 mixes.

**Table 4.** Comparison of the research results conducted with the elasticity modulus of different codes.

212 4.6. Modeling the modulus of elasticity of crumb rubber concrete mixes

213 A model for estimating the modulus of elasticity of crumb rubber concrete mixes was presented following  
214 the formula using mini-tab software and laboratory data (1).

215

$$E_c = 9.25 + 3.835\sqrt{F_c} \quad \text{Equation 1}$$

216  $E_c$ : Modulus of elasticity (Gpa)

217  $F_c$ : Compressive strength (Mpa)

218 To better understand the process in this research and check the model obtained, a comparison of the  
219 laboratory results (E), and the final solution of this model (Ec) is presented in figures 11-12.

Figure 11. The relationship of Ec in this research with the research of others [82–84].

Figure 12. Comparison of the laboratory results (E) and the final solution of this model (Ec) in this  
research with the research of others [82, 84, 85].

#### 220 4.7. Ultrasonic wave speed

221 Figure 13 depicts the method of altering the speed of ultrasonic waves in 10 \* 10 \* 10 cm concrete  
222 specimens as the proportion of crumb rubber varies. The transmission speed is reduced when crumb  
223 rubber is added. In a body with high porosity, the wave velocity is low. When the porosity is low, the  
224 wave velocity is high. The results exhibited that the ultrasonic reduced as the amount of rubber increased.  
225 A 12%, 40%, and 65% reduction was observed in ultrasonic when 5%, 10%, and 15% coarse aggregate  
226 was substituted with rubber aggregate at 28 days compared to the control mix samples. An ultrasonic  
227 improvement was observed by adding pozzolan to the samples, as shown in figure 14.

Figure 13. Ultrasonic of crumb rubber concrete at 7 and 28 days.

Figure 14. Ultrasonic of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

228 4.8. Electrical resistivity test

229 7-day and 28-day electrical resistivity of waste rubber concrete is shown in figure 15. Electrical resistance  
230 rises as the amount of shredded rubber in the mixture rises. Because rubber is a dielectric material, rubber  
231 particles in the concrete act as an insulator, stopping electricity from flowing between the two measuring  
232 electrodes [60]. An important change in the increased electrical resistivity was noticed [45], as shown in  
233 figure 16.

Figure 15. Electrical resistivity of crumb rubber concrete at 7 and 28 days.

Figure 16. Electrical resistivity of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

234 4.9. Water absorption test and Porosity test

235 The amount of rubber in the concrete impacts its porosity, which has an effect on its water absorption  
236 capacity [29]. As the percentage of shredded rubber in concrete was raised, the porosity and water  
237 absorption increased Table 5. Because rubber particles are non-polar, air bubbles can be trapped on their  
238 surfaces. As a result, the cement-aggregate interface becomes more porous and absorbent. According to  
239 Mohammed et al. [29], replacing 10% of the cement with silica fumes results in more than un-reacted  
240 silica fume in the matrix. Due to its micro filling capabilities, this can fill air gaps within the rubber  
241 concrete's microstructure, reducing water absorption [45]. Adding silica fumes to the concrete samples  
242 helped reduce porosity and water absorption Table 4.

**Table 5.** Results of % water absorption test and % porosity.

243 **5. Discussions**

244 Slump decreased by 3%, 7%, and 15% increasing the aggregate replacement with crumb rubber by 5 %,  
245 10%, and 15%, respectively, compared to the control mix samples. The slump was improved by adding  
246 silica fume and zeolite to the rubber concrete mix. Replacing 15% rubber with aggregate containing 10%  
247 silica fume showed that the compressive strength rose by 47% compared to CR15%. A reduction of 1%  
248 and 4%, in flexural strength was observed when 10% and 15% coarse aggregate was substituted with  
249 rubber aggregate, respectively, to the control mix samples. They were improved by adding silica fume  
250 and zeolite.

251 Compressive strength was reduced by 20%, and 36% at 7 days after increasing the aggregate replacement  
252 with crumb rubber by 10%, and 15%, respectively, in comparison to the control mix samples. They  
253 were improved by adding silica fume and zeolite. It is possible to explain the reduction in  
254 compression strength of concrete incorporating tire rubber particles by three major factors:

- 255 • In contrast to the surrounding cement paste, rubber particles are more deformable, which results  
256 in cracks close to the rubber particles, which resemble cracks that appear around air voids in  
257 typical concrete [28].
- 258 • As a result of the cement paste and crumb rubber's poor adhesion [86].
- 259 • Last but not least, due to the potential of a diminution in concrete matrix density, which is a  
260 function of aggregate density, size, and hardness.

261 Compressive strength of rubber concrete is assumed to be influenced more by the deformability and soft  
262 aggregate-like properties of tire rubber particles than by either of the other two effects. The observation  
263 that tire rubber particles of different sizes appear to have a considerable influence on the compressive  
264 strength of a tire lends support to this theory. As tire rubber particles become larger, rubber concrete's  
265 compressive strength decreases. As a result, tire rubber particles can be described as soft aggregate

266 components that develop tensile stresses at their surfaces and next to the cement paste when the rubber  
267 concrete mix is put under compressive stress [25].

268 Flexural strength decreased by 15% and 29%, and Tensile strength declined by 13% and 22% when  
269 10% and 15% coarse aggregate were substituted with rubber aggregate, respectively, at the age of 7 days  
270 compared to the control mix samples. Tensile strength and flexural strength were improved by adding  
271 silica fume and zeolite to the rubber concrete mix.

272 A reduction of 5%, 8%, and 15% at 28-day was observed in the modulus of elasticity (E) when 5%, 10%,  
273 and 15% coarse aggregate was substituted with rubber aggregate, respectively. The modulus of elasticity  
274 was improved by adding silica fume and zeolite to the rubber concrete mix.

275 Ec decreased by 2% and 7% when 5% and 10% coarse aggregate was substituted with rubber aggregate,  
276 respectively, compared to the control mix samples. There is a direct relationship between Ec and E. As Ec  
277 decreased, E also decreased. This study shows the relationship of Ec in this research with the research of  
278 others. Gupta and other researchers [84], showed a reduction of 23% and 11% in Ec when 5%, 10%, and  
279 aggregate were substituted with rubber aggregate, respectively.

280 A 13% and 41% reduction was observed in ultrasonic when 5% and 10% coarse aggregate was  
281 substituted with rubber aggregate at 7 days compared to the control mix samples. An ultrasonic  
282 improvement was observed by adding pozzolan to the samples.

283 Electrical resistivity rose by 7%, and 26% at 7 days after increasing the aggregate replacement with  
284 crumb rubber by 5 %, and 10%, respectively, in comparison to the control mix samples. By replacing  
285 15% rubber with aggregate containing 10% silica fume and zeolite, the electrical resistivity increased by  
286 32% and 36% on days 7, respectively, contrasted to CR15%.



287 Water absorption increased by 1% and 22%, and porosity rose by 8%, and 20% at 28 days after increasing  
288 the aggregate replacement with rubber crumbs by 5%, and 10%, respectively, in comparison to the control  
289 mix samples. The samples were improved by adding silica fume and zeolite.

## 290 **6. Conclusion**

291 This paper investigated the mechanical properties of concrete containing crumb rubber and mineral  
292 additives silica fume and zeolite. The following items can be concluded according to this article's  
293 parameters and values.

- 294 1. In this research, the workability of rubber concrete mix reduced when the quantity of crumb  
295 rubber rose.
- 296 2. When using crumb rubber concrete, the compressive strength diminishes as the quantity of  
297 shredded rubber rises, it reduces the adhesive between crumb rubber and the cement, resulting in  
298 quick concrete rupture during loading. Also, tensile strength showed a reduction with a rise in the  
299 percentage of rubber crumbs content. However, the strength improves when zeolite and silica  
300 fume are used in place of some of the cement.
- 301 3. When the quantity of shred rubber in the mix increases, the flexural strength decreases because  
302 fissures exist, causing poor bonding between the constituent materials. The strength of a structure  
303 improves when zeolite and silica fume are used in place of some of the cement.
- 304 4. The ultrasonic testing results indicated that the porosity in concrete containing crumb rubber  
305 increased; thus, the modulus of elasticity decreased with rising porosity. In rubberized concrete  
306 mixes, a decrease in static modulus of elasticity suggests increased flexibility, which may be  
307 considered a good gain.

308 5. The water absorption of concrete containing rubber particles rises as the substitution amount  
309 increases. Greater water penetration has resulted from the formation of voids and fractures  
310 because of the larger surface area of shredded rubber.

### 311 **COI Statement**

312 The authors of this study declare that they have no conflict of interest.

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509 **Biographies**

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- 533 Figure 1. Compressive strength of crumb rubber concrete at 7 and 28 days.
- 534 Figure 2. Compressive strength of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.
- 535 Figure 3. Flexural strength of crumb rubber concrete at 7 and 28 days.
- 536 Figure 4. Flexural strength of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.
- 537 Figure 5. Tensile strength of crumb rubber concrete at 7 and 28 days.
- 538 Figure 6. Tensile strength of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.
- 539 Figure 7. Relationship among 28-day compressive strength and 28 days tensile strength of concrete  
540 containing shredded rubber, silica fume, zeolite.
- 541 Figure 8. Modulus of elasticity of crumb rubber concrete at 7 and 28 days.
- 542 Figure 9. Modulus of elasticity of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.
- 543 Figure 10. Relation between Modulus of elasticity and 28 days Porosity of concrete containing crumb  
544 rubber, silica fume, zeolite.
- 545 Figure 11. The relationship of  $E_c$  in this research with the research of others.
- 546 Figure 12. comparison of the laboratory results ( $E$ ) and the final solution of this model ( $E_c$ ) in this  
547 research with the research of others
- 548 Figure 13. Ultrasonic of crumb rubber concrete at 7 and 28 days.
- 549 Figure 14. Ultrasonic of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.
- 550 Figure 15. Electrical resistivity of crumb rubber concrete at 7 and 28 days.
- 551 Figure 16. Electrical resistivity of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.
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- 552 **Table 1.** Cement's physical and chemical characteristics and pozzolans.
- 553 **Table 2.** Proportions of crude rubber and pozzolan in concrete containing crude rubber and pozzolan.
- 554 **Table 3.** Slump test results and specific weight.
- 555 **Table 4.** Comparison of the research results conducted with the elasticity modulus of different codes.
- 556 **Table 5.** Results of % Water absorption test and % porosity.

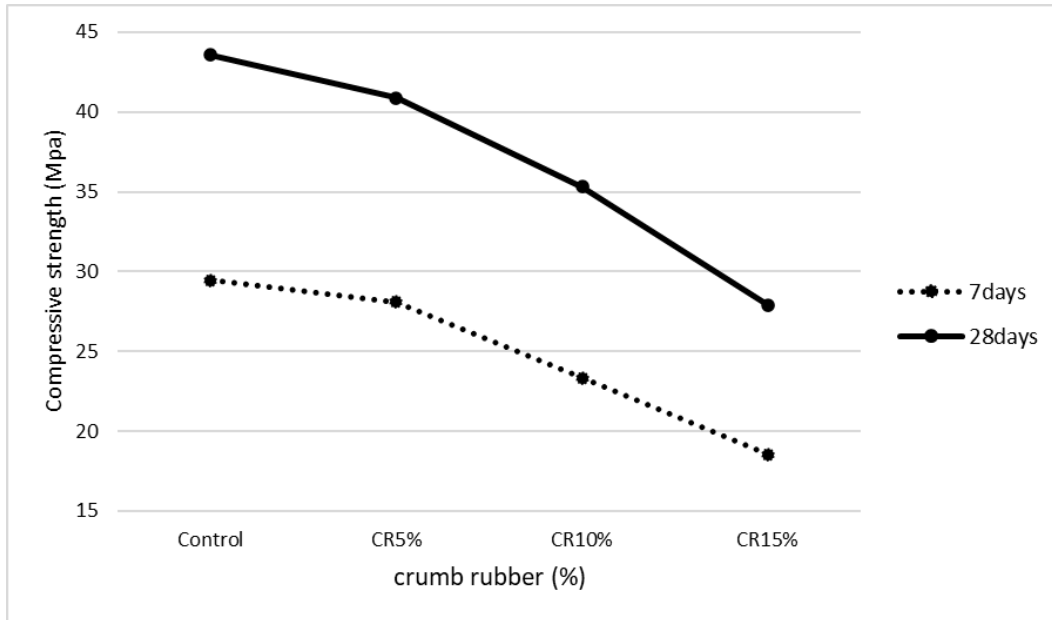


Figure 1. Compressive strength of crumb rubber concrete at 7 and 28 days.

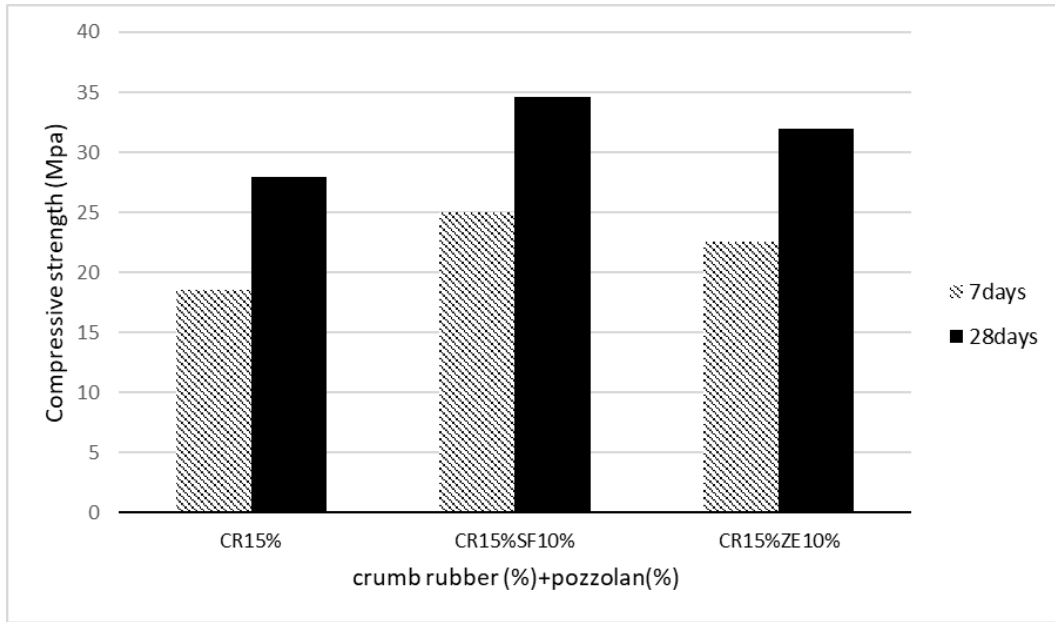


Figure 2. Compressive strength of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

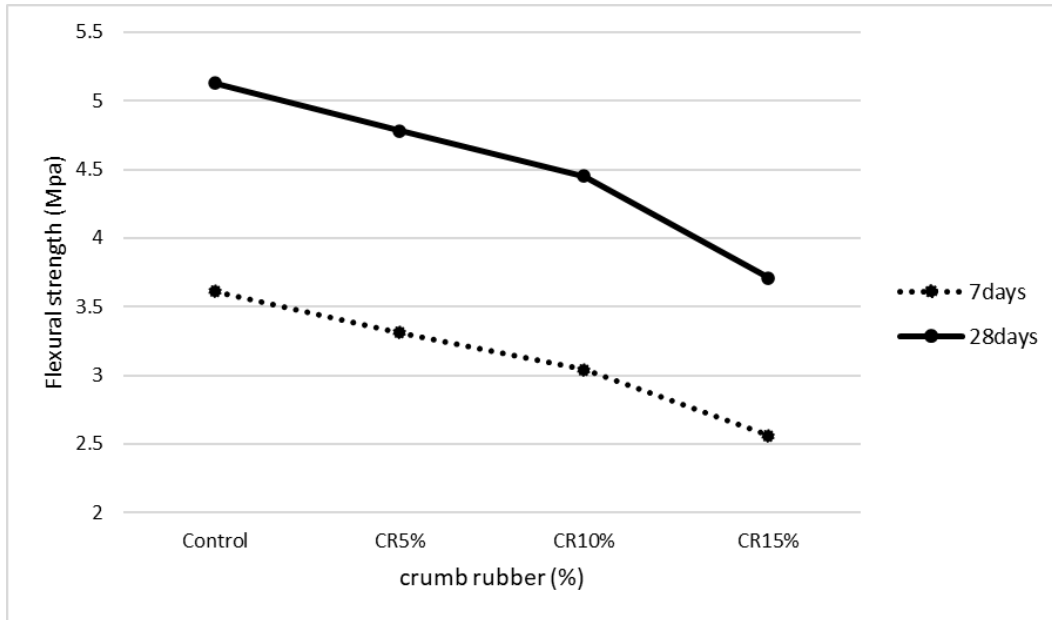


Figure 3. Flexural strength of crumb rubber concrete at 7 and 28 days.

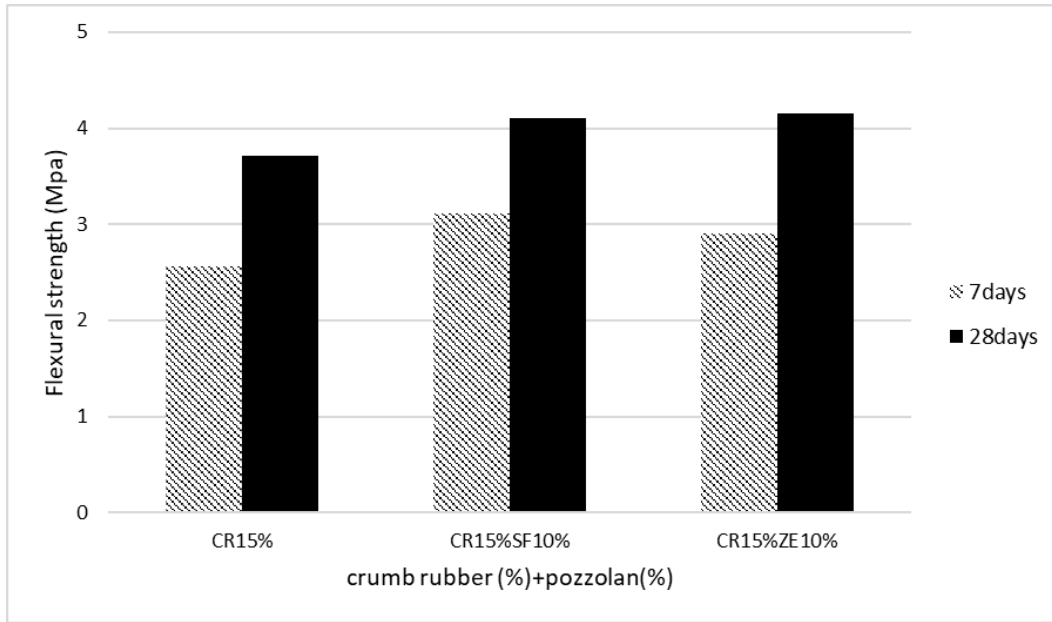


Figure 4. Flexural strength of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

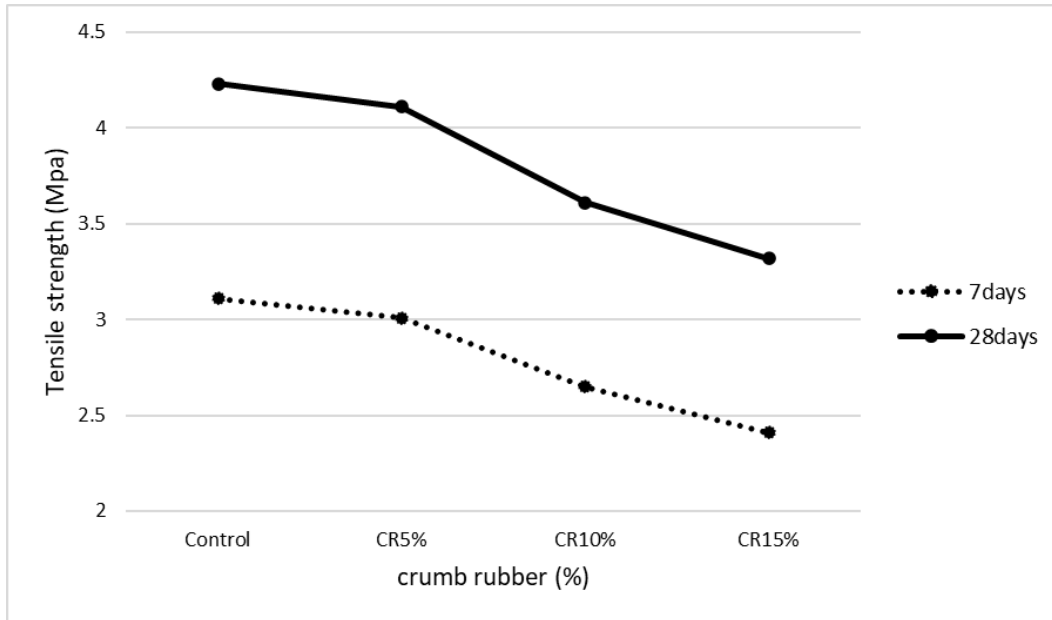


Figure 5. Tensile strength of crumb rubber concrete at 7 and 28 days.

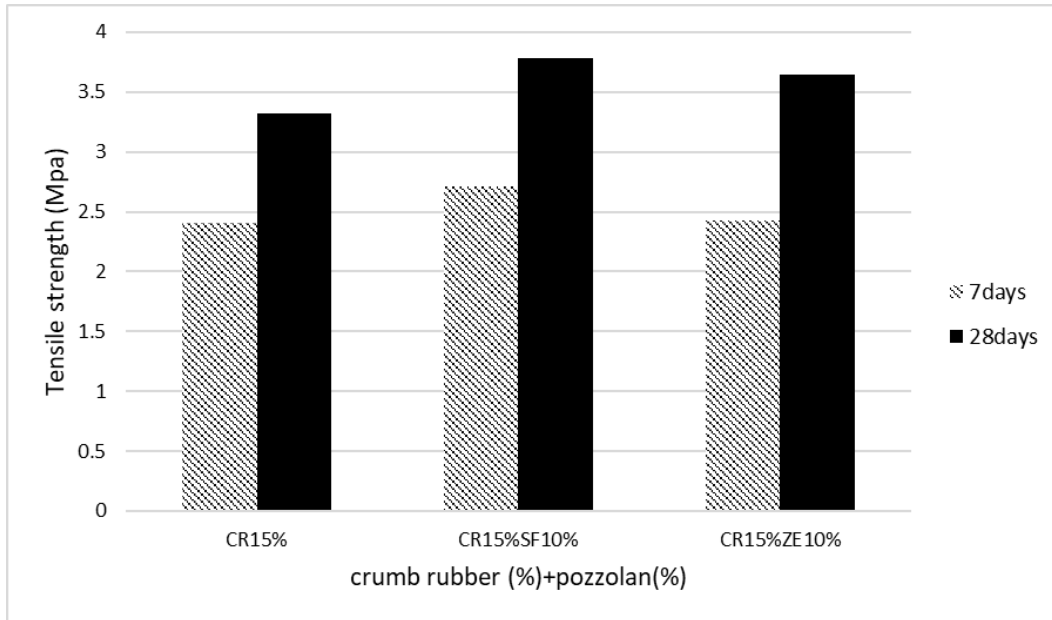


Figure 6. Tensile strength of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

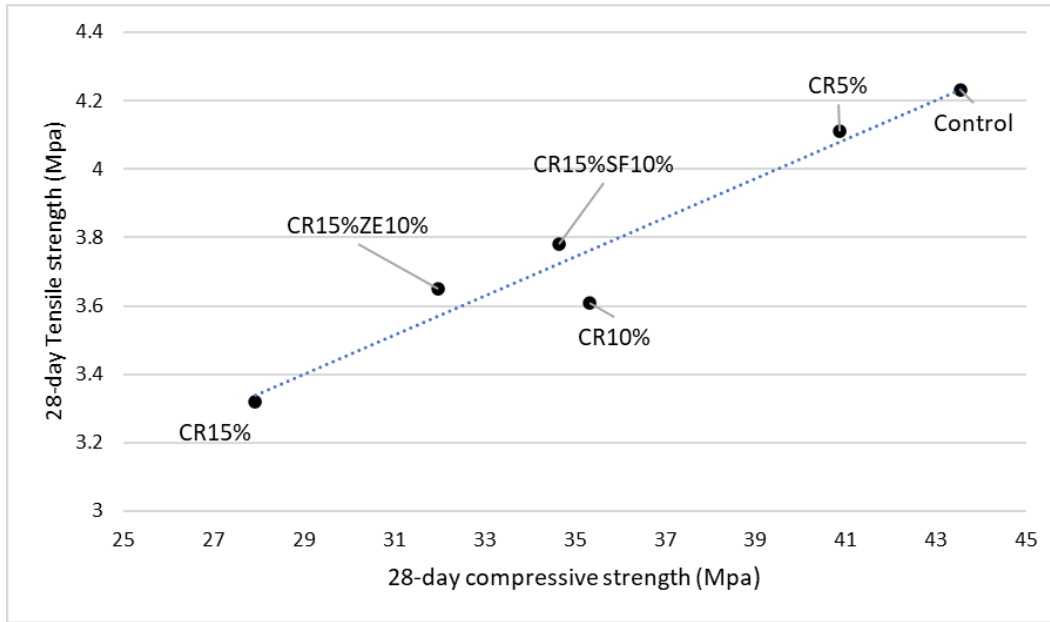


Figure 7. Relationship among 28-day compressive strength and 28 days tensile strength of concrete containing shredded rubber, silica fume, zeolite.



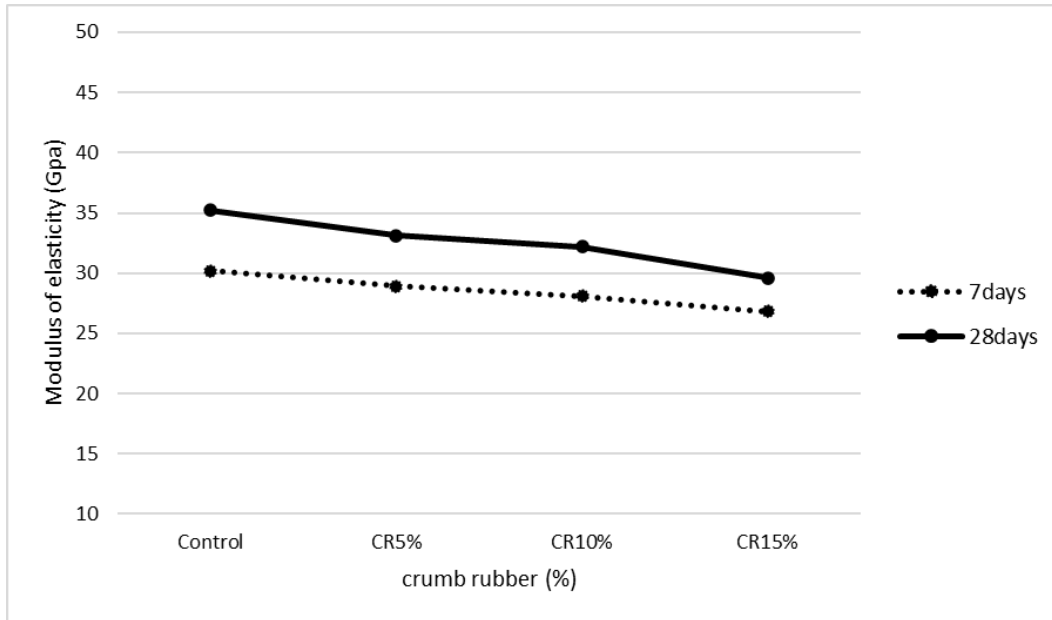


Figure 8. Modulus of elasticity of crumb rubber concrete at 7 and 28 days.

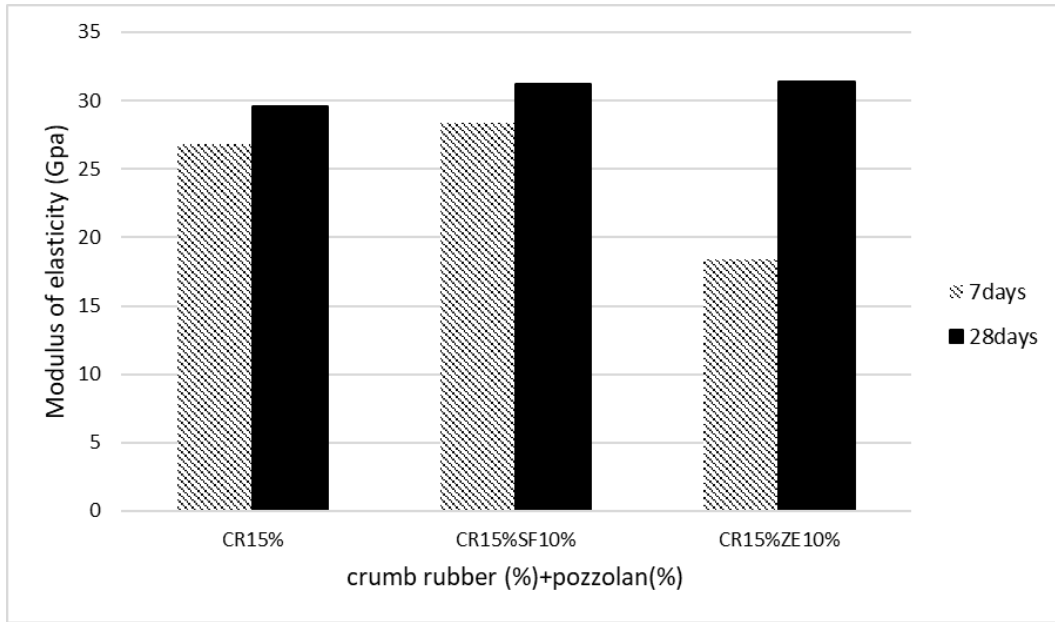


Figure 9. Modulus of elasticity of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

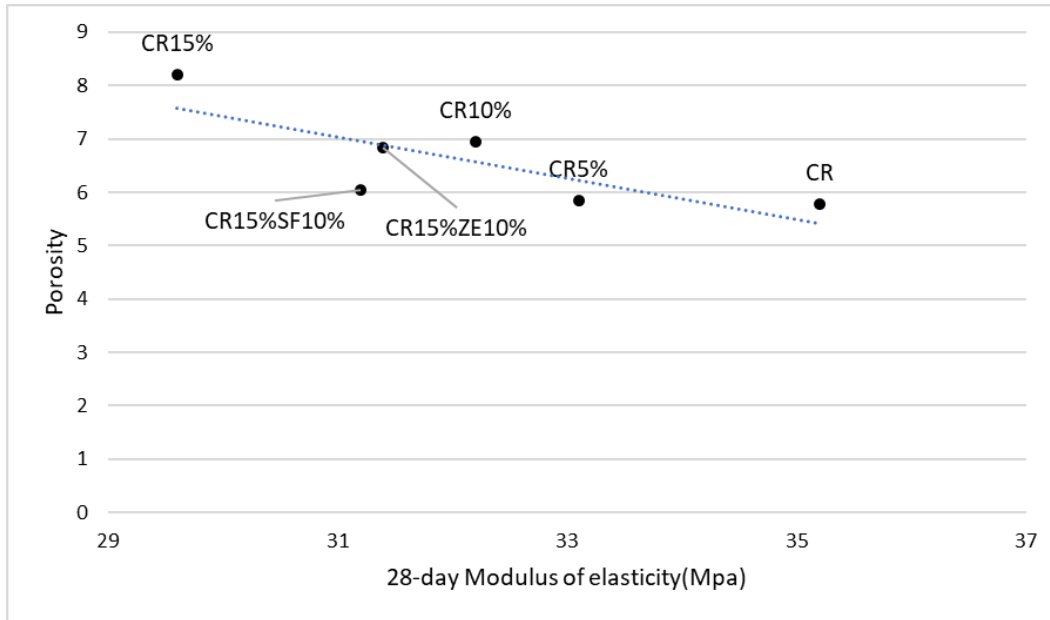


Figure 10. Relation between Modulus of elasticity and 28 days Porosity of concrete containing crumb rubber, silica fume, zeolite.

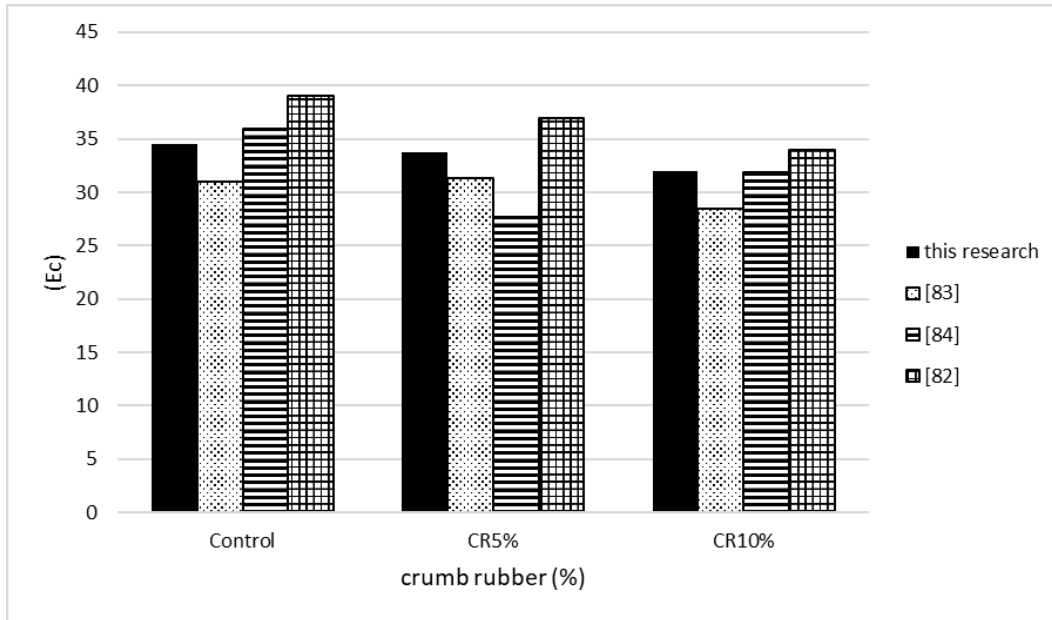


Figure 11. The relationship of Ec in this research with the research of others [82–84].

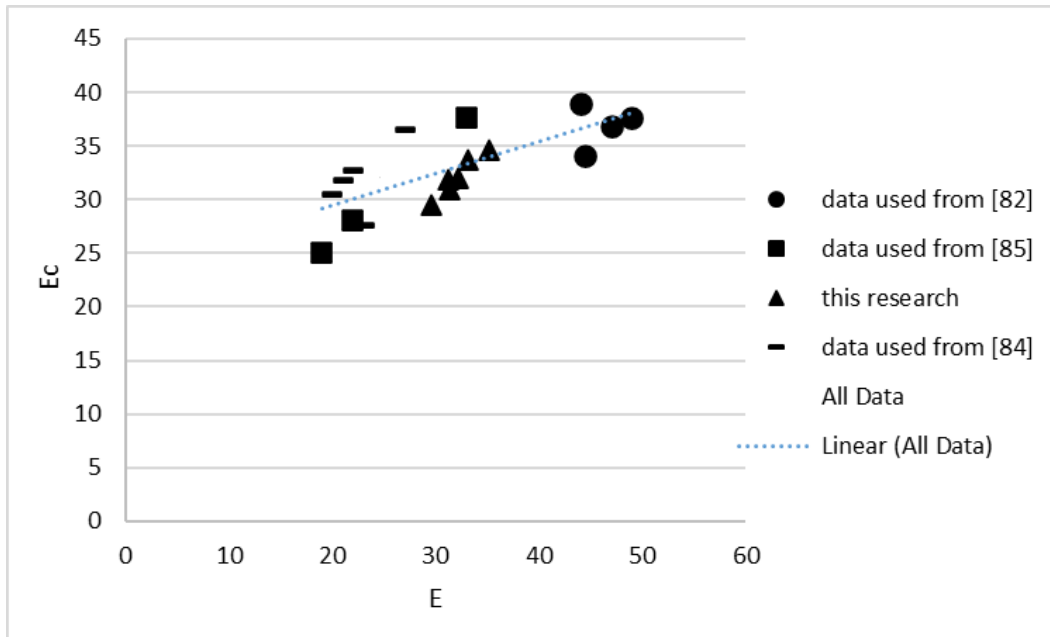


Figure 12. comparison of the laboratory results (E) and the final solution of this model (Ec) in this research with the research of others [82,84,85].

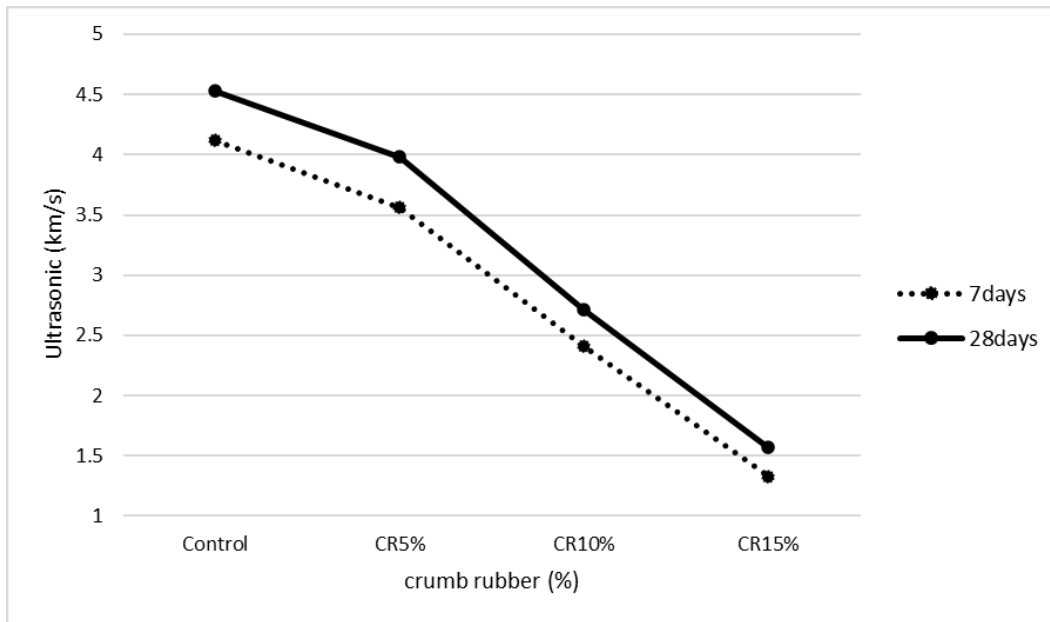


Figure 13. Ultrasonic of crumb rubber concrete at 7 and 28 days.

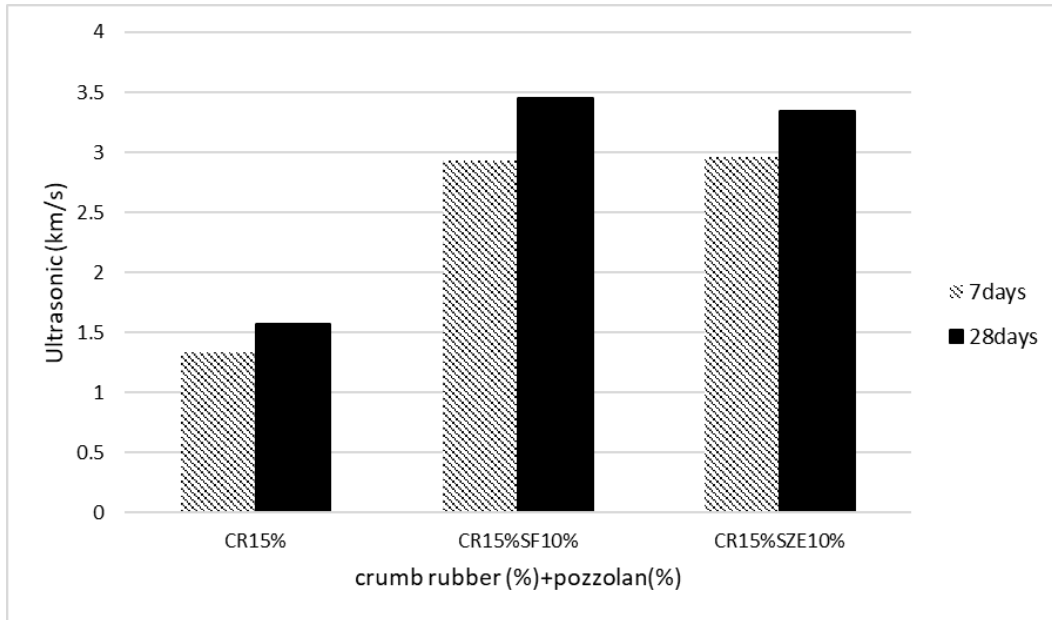


Figure 14. Ultrasonic of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

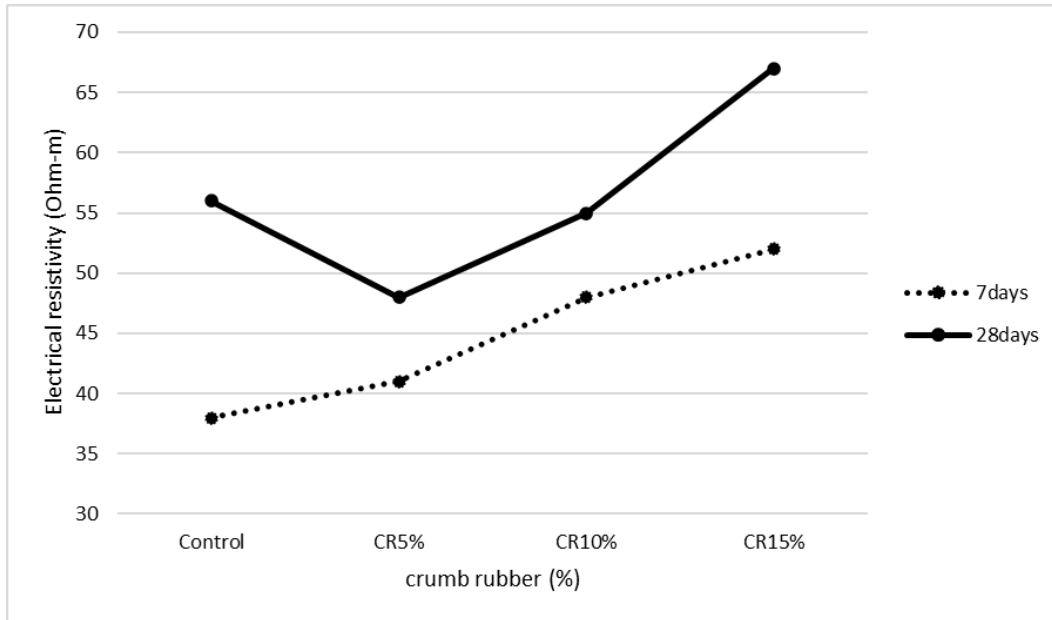


Figure 15. Electrical resistivity of crumb rubber concrete at 7 and 28 days.



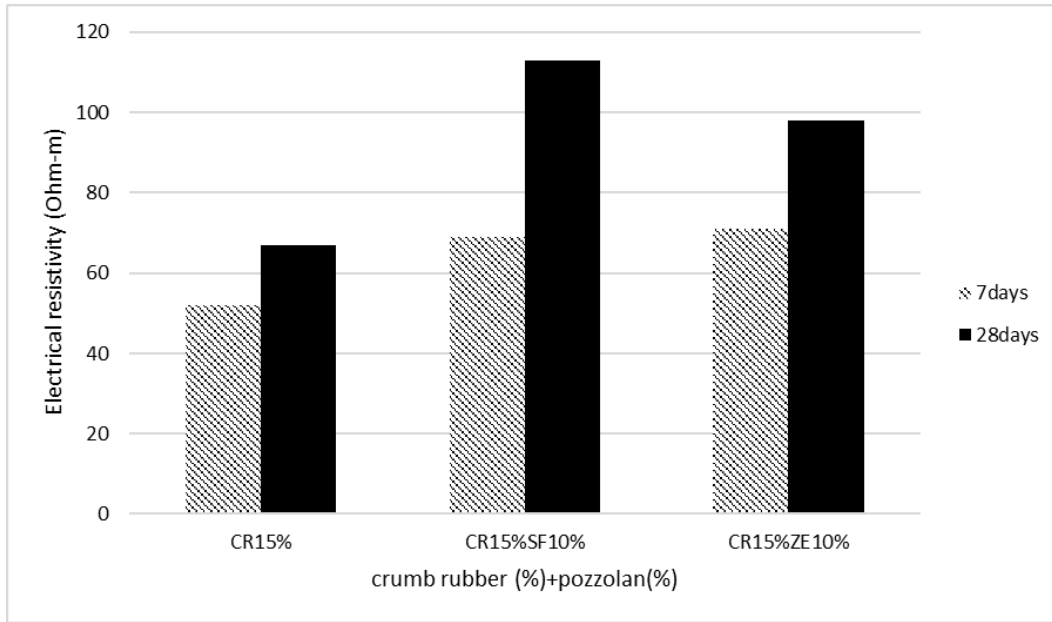


Figure 16. Electrical resistivity of crumb rubber concrete at 7 and 28 days + silica fume and zeolite.

**Table 1.** Cement's physical and chemical characteristics and pozzolans.

Chemical analyses	Portland Cement (%)	silica fume (%)	Zeolite (%)
SiO <sub>2</sub>	20.74	87.49	66.5
Al <sub>2</sub> O <sub>3</sub>	4.90	2.87	11.81
Fe <sub>2</sub> O <sub>3</sub>	3.50	1.27	1.3
CaO	62.95	1.55	3.11
MgO	1.20	1.31	0.72
Na <sub>2</sub> O	-	0.38	2.01
K <sub>2</sub> O	-	0.41	3.12
SO <sub>3</sub>	3.00	0.17	-
Loss of ignition	1.56	-	-
Insoluble residue	0.74	-	-
Specific gravity(kg/m <sup>3</sup> )	3050	-	-
Specific surface area(kg/m <sup>3</sup> )	2805	-	-

**Table 2.** Proportions of crude rubber and pozzolan in concrete containing crude rubber and pozzolan.

Mix ID	Cement (Kg)	Silica fume (kg)	Zeolite (kg)	Coarse aggregate (kg)	Fine aggregate (kg)	Sand (kg)	Rubber (kg)	W/C	Water (kg)
Control	420	0	0	547.68	544.83	707.17	0	0.4	225.88
CR5%	420	0	0	520.55	516.68	707.17	22.81	0.4	225.88
CR10%	420	0	0	493.15	489.49	707.17	45.62	0.4	225.88
CR15%	420	0	0	465.76	462.29	707.17	68.44	0.4	225.88
CR15%SF10%	324.70	95.3	0	465.76	462.29	707.17	68.44	0.4	225.88
CR15%ZE10%	306.61	0	113.39	465.76	462.29	707.17	68.44	0.4	225.88

**Table 3.** Slump test results and specific weight.

Mix ID	Slump (cm)	Specific weight (kg/m <sup>3</sup> )
Control	5.7	2391
CR5%	5.5	2368
CR10%	5.3	2354
CR15%	4.8	2284
CR15%SF10%	2.5	2307
CR15%ZE10%	4.6	2324

**Table 4.** Comparison of the research results conducted with the elasticity modulus of different codes.

Mix ID	Modulus of elasticity was performed	ACI 318-08[87]	CSA A23.3[76]	NBR 6118[77]
		$E_c$ $= 4730(f'c)^{0.5}$	$E_c$ $= 4500(f'c)^{0.5}$	$E_c$ $= 5600(f'c)^{0.5}$
Control	35.2	31.21	29.7	36.96
CR5%	33.1	30.23	28.76	35.8
CR10%	32.2	28.11	26.74	33.28
CR15%	29.6	24.99	23.78	29.59
CR15%SF10%	31.2	27.84	26.48	32.96
CR15%ZE10%	31.4	26.74	25.44	31.66

**Table 5.** Results of % Water absorption test and % porosity.

Mix ID	% Water absorption	% porosity
Control	2.81	5.77
CR5%	2.85	5.85
CR10%	3.44	6.94
CR15%	4.19	8.20
CR15%SF10%	2.98	6.05
CR15%ZE10%	3.44	6.84