Axial translation of a rigid disc inclusion embedded in a penny-shaped crack in a transversely isotropic solid

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**Abstract.** This paper investigates an analytical solution for the axisymmetric interaction of rigid disc inclusion embedded in bonded contact with the surfaces of a penny-shaped crack and a transversely isotropic medium. By using a method of potential functions and treating dual and triple integral equations, the mixed boundary value problem is written in the form of two coupled integral equations, which are amenable to numerical treatments. The axial stiffness of the inclusion and the shearing stress intensity factor at the tip of the penny-shaped crack for different degrees of material anisotropy are illustrated graphically. Useful limiting cases such as a rigid disc inclusion in an uncracked medium and in a completely cracked solid are recovered.

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1. Introduction

The category of problems that examines the mechanical behavior of contact regions constitutes an important branch of applied mechanics with extensive engineering applications [1]. Nowadays, composites play a very important role in geomechanical engineering. It is common knowledge that all existing structural materials contain different inter- and intra-component defects (cracks, delaminations, etc.) [2]. Analysis of interaction between cracks and inclusion has important applications to the study of micro-mechanics of multi-phase materials and to the examination of anchoring devices embedded in geological media [3].

studied the axial loading of an annular crack by a rigid disc inclusion and presented the shearing stress intensity factor and the axial stiffness of the inclusion for different ratios of the inner and outer radii of the annular crack. Vrbik et al. [14] studied the symmetric indentation of a penny-shaped crack by a smoothly embedded rigid circular disc inclusion in a thick layer.

Recently, Eslandari et al. [15] discussed separation of dissimilar piezoelectric half-spaces by a rigid disc inclusion. Shodja et al. [16] analyzed the interaction of the annular and penny-shaped cracks in an infinite piezoelectric medium (see also [17]). Singh et al. [18] performed the study of indentation of an elliptic crack by a rigid elliptic inclusion in the anti-plane shear mode. Eslandari-Ghadi et al. [19] presented a mathematical formulation for the analysis of a transversely isotropic half-space containing a disc-shaped crack buried at an arbitrary depth. Fabrikant [20] considered a transversely isotropic body weakened on the plane perpendicular to the planes of isotropy of the transversely isotropic body (see also [21-23]). Yang and Zhao [24] investigated the influence of a capillary bridge or a liquid droplet on the crack opening and stress intensity factor of a penny-shaped crack under a far-field tensile stress. Shahlomamadi et al. [25] studied the axial interaction of a rigid disc with a penny-shaped crack in a transversely isotropic full-space. Moreover, Antipov and Mkhitararyan [26] studied the plane problem of interaction between a thin rigid inclusion and a finite crack. Amiri-Hezaveh et al. [27] examined the dynamic indentation of a rigid circular plate embedded in a non-homogeneous transversely isotropic full-space. Eslandari-Ghadi et al. [28] presented an analytical solution for a two-layer transversely isotropic half-space containing a penny-shaped crack located at the interface of layers.

In this paper, the main objective is to investigate the interaction of a rigid disc in bonded contact with the surfaces of a penny-shaped crack and an infinite transversely isotropic medium. By virtue of appropriate displacement-potential functions and Hankel and Abel transforms, the solution of the problem is reduced to two coupled Fredholm integral equations, which are amenable to numerical treatments. The axial stiffness of the inclusion and the mode II stress intensity factor at the tip of the crack are obtained for several types of hypothetical transversely isotropic materials.

From a practical viewpoint, in geomechanical applications, the rigid disc-shaped inclusion represents the behavior of an earth or rock anchor that is created by the hydraulic fracture of the earth or rock mass. The inclusion represents the resinous or cementing material that is used to transfer anchoring loads to the geological medium [29].

2. Statement of the problem and the governing equations

Consider a rigid circular disc inclusion of radius \( a \) surrounded by a penny-shaped crack of radius \( b \) in an infinite transversely isotropic medium while the disc is in perfect contact with the surfaces of the crack (see Figure 1). The disc is subjected to a central force \( T \) which induces a rigid-body displacement \( \delta \) in \( z \) direction. Because of symmetry, it suffices to limit attention to one half-space \( (0 \leq z < \infty) \). The mixed boundary conditions of the problem under consideration in terms of displacement vector \( \mathbf{u} \) and Cauchy stress tensor \( \sigma \) are as follows:

\[
\begin{align*}
  u_z(r, 0) &= \delta, \quad 0 \leq r \leq a, \\
  u_z(r, 0) &= 0, \quad 0 \leq r \leq a, \quad b \leq r < \infty, \\
  \sigma_{zz}(r, 0) &= 0, \quad a < r < \infty, \\
  \sigma_{rr}(r, 0) &= 0, \quad a < r < b.
\end{align*}
\]

For axisymmetric problems, the equilibrium equations of the static motion for a homogeneous transversely isotropic elastic solid in terms of displacement and in the absence of the body forces can be expressed as follows [30]:

\[
c_{11} \left( \frac{\partial^2 u_r}{\partial r^2} + \frac{1}{r} \frac{\partial u_r}{\partial r} - \frac{u_r}{r^2} \right) + c_{44} \frac{\partial^2 u_r}{\partial z^2} + (c_{13} + c_{44}) \frac{\partial^2 u_r}{\partial r \partial z} = 0,
\]

\[
\begin{figure}[h]
\centering
\includegraphics[width=0.8\textwidth]{Figure_1.png}
\caption{Axial translation of a rigid disc inclusion embedded in a penny-shaped crack.}
\end{figure}
where $u_r$ and $u_z$ are the displacement components in $r$ and $z$ directions, respectively, and $c_{ij}$ represents the elastic constants of the solid. The displacement and stress fields for a semi-infinite transversely isotropic medium ($0 \leq z < \infty$) are as follows [31]:

$$ u_z(r, z) = \int_0^\infty -\xi^3 \left[ A'(\xi) (1 + \alpha_1 - s_1^2 \alpha_2) e^{-\xi z} + B'(\xi) (1 + \alpha_1 - s_2^2 \alpha_2) e^{-\xi z} \right] J_0(\xi r) d\xi,$$

$$ u_r(r, z) = \int_0^\infty -\xi^3 \alpha_2 \left[ A'(\xi) s_1 e^{-\xi z} + B'(\xi) s_2 e^{-\xi z} \right] J_1(\xi r) d\xi,$$

$$ \sigma_{zz}(r, z) = \int_0^\infty -\xi^4 \left[ A'(\xi) s_1 e^{-\xi z} + B'(\xi) s_2 e^{-\xi z} \right] \left[ c_{33} \alpha_2 (s_1^2 + s_2^2) + (c_{13} + c_{44}) (1 + \alpha_1) \right] J_0(\xi r) d\xi,$$

$$ \sigma_{rr}(r, z) = \int_0^\infty -\xi^4 c_{44} \left[ A'(\xi) (1 + \alpha_1 + s_1^2 (\alpha_3 - \alpha_2)) + B'(\xi) (1 + \alpha_1 + s_2^2 (\alpha_3 - \alpha_2)) \right] e^{-\xi z} \left[ J_1(\xi r) d\xi \right],$$

where $A'(\xi)$ and $B'(\xi)$ are unknown functions and

$$ \alpha_1 = \frac{c_{12} + c_{66}}{c_{66}}, \quad \alpha_2 = \frac{c_{44}}{c_{66}}, \quad \alpha_3 = \frac{c_{13} + c_{44}}{c_{66}}.$$

Herein, $s_1$ and $s_2$ are the roots of the following equation that, in view of the positive-definiteness of the strain energy, are not zero or pure imaginary numbers [30]:

$$ c_{33} c_{44} s_1^4 - \left[ c_{13}^2 + 2 c_{12} c_{44} - c_{11} c_{33} \right] s_1^2 + c_{11} c_{44} = 0.$$

Introducing the substitutions:

$$ [A(\xi); B(\xi)] = \frac{1}{\xi^3} [A'(\xi); B'(\xi)],$$

and using Eq. (6), boundary conditions (1)-(4) can be reduced to the following system of integral equations:

$$ \int_0^a M(\xi) J_0(\xi r) d\xi = \delta, \quad 0 \leq r \leq a, \quad (10)$$

$$ \int_0^a N(\xi) J_1(\xi r) d\xi = \gamma_1 \int_0^a M(\xi) J_1(\xi r) d\xi, \quad 0 \leq r \leq a, \quad b \leq r < \infty, \quad (11)$$

$$ \int_0^a \xi M(\xi) J_0(\xi r) d\xi = \gamma_2 \int_0^a \xi N(\xi) J_0(\xi r) d\xi, \quad a < r < \infty, \quad (12)$$

$$ \int_0^a \xi N(\xi) J_1(\xi r) d\xi = 0, \quad a < r < b, \quad (13)$$

where $\gamma_k$ ($k = 1, 2$) and functions $M(\xi)$ and $N(\xi)$ are mentioned in Appendix A.

### 2.1. Dual integral equations

By making use of Eqs. (10) and (12) the following system of dual integral equations is obtained:

$$ \int_0^a M(\xi) J_0(\xi r) d\xi = \delta, \quad 0 \leq r \leq a, \quad (14)$$

$$ \int_0^a \xi M(\xi) J_0(\xi r) d\xi = \gamma_2 \int_0^a \xi N(\xi) J_0(\xi r) d\xi, \quad a < r < \infty. \quad (15)$$

The integral equations (14) and (15) yield the following:

$$ M(\xi) = \frac{2}{\pi} \left[ \frac{\delta \sin(\xi a)}{\xi} + \int_0^\infty F(u) \cos(\xi u) du \right], \quad (16)$$

where:

$$ F(t) = \gamma_2 \int_0^a N(\xi) \cos(\xi t) d\xi. \quad (17)$$

### 2.2. Triple integral equations

By making use of Eqs. (11) and (13), the following system of triple integral equations is obtained:

$$ \int_0^b N(\xi) J_1(\xi r) d\xi = 0, \quad a < r < b, \quad (19)$$

$$ \int_0^b \xi N(\xi) J_1(\xi r) d\xi = \gamma_1 \int_0^b \xi M(\xi) J_1(\xi r) d\xi, \quad b \leq r < \infty. \quad (20)$$

Taking the following assumption:
$$\int_0^\infty \xi N(\xi) J_i(\xi r) d\xi = \begin{cases} f_1(r), & 0 < r < a \\ 0, & a < r < b \\ f_2(r), & b < r < \infty \end{cases}$$ (21)

By employing the inverse Hankel integral transform to Eq. (21), the following relation is obtained:

$$N(\xi) = \int_0^a r f_1(r) J_i(\xi r) \, dr + \int_b^\infty r f_2(r) J_i(\xi r) \, dr.$$ (22)

Inserting Eq. (22) into Eq. (11), we have:

$$I_1(r) + I_2(r) = g(r), \quad 0 < r < a,$$ (23)

$$I_1(r) + I_2(r) = g(r), \quad b < r < \infty,$$ (24)

where:

$$I_j(r) = \int \lambda f_j(\lambda) \mathcal{L}(r, \lambda) \, d\lambda, \quad (j = 1, 2),$$ (25)

$$L(r, \lambda) = \int_0^\infty J_i(\xi r) J_i(\xi \lambda) \, d\xi,$$ (26)

$$g(r) = \gamma_1 \int_0^\infty M(\xi) J_i(\xi r) \, d\xi,$$ (27)

and the limits of integration in Eq. (25) can occupy (0, a) and (b, \infty) ranges depending upon the value of j.

Using the procedure presented in [32], Eqs. (23) and (24) can be rewritten as follows:

$$\int_0^r F_1(s) \, ds = -r^2 \int_b^\infty \frac{F_2(s) \, ds}{s^2 (s^2 - r^2)^{1/2}} - \frac{\pi r^2}{2} g(r), \quad 0 < r < a,$$ (28)

$$\int_r^\infty \frac{F_2(s) \, ds}{s^2 (s^2 - r^2)^{1/2}} = -\frac{1}{r^2} \int_b^a \frac{F_1(s) \, ds}{s^2 (s^2 - r^2)^{1/2}} + \frac{\pi r^2}{2} g(r), \quad b < r < \infty,$$ (29)

where:

$$F_1(s) = s^2 \int_0^a \frac{f_1(u) \, du}{(u^2 - s^2)^{1/2}},$$ (30)

$$F_2(s) = \int_b^\infty \frac{u^2 f_2(u) \, du}{(s^2 - u^2)^{1/2}}.$$ (31)

Insertion of Eqs. (36) and (37) into Eqs. (32) and (33) yields the following coupled Fredholm integral equations:

$$F_1(s) = -\frac{2}{\pi^2} \gamma_1 \gamma_2 \int_0^a \frac{s F_1(u)}{u (u^2 - s^2)} \left[ u \ln \frac{a - u}{a + u} - 2 \frac{s^2}{u (u^2 - s^2)} \right] F_2(u) \, du + \frac{s^2}{2 a^2} \ln \frac{s + a}{s - a} F_2(u) \, du,$$ (32)

$$F_2(u) = 2 \pi \gamma_1 \gamma_2 \int_b^\infty \frac{s (u - a) (a + s)}{u (u^2 - s^2)} \left[ \frac{s^2}{2 a^2} \ln \frac{u - a}{u + a} - \frac{s^2}{u (u^2 - s^2)} \right] F_2(u) \, du + \frac{s^2}{2 a^2} \ln \frac{s + a}{s - a} F_2(u) \, du,$$ (33)

where:

$$F_1(s) = -\frac{2}{\pi} \int_b^\infty \left[ \frac{s^2}{u (u^2 - s^2)^2} + \frac{s}{2 u^2} \ln \frac{s + u}{s - u} \right] F_2(u) \, du,$$ (34)

$$F_2(s) = -\frac{s^2}{2 a^2} \ln \frac{s + a}{s - a} F_2(u) \, du + \frac{s}{2 u^2} \ln \frac{s + a}{s - a} F_2(u) \, du,$$ (35)

and Eqs. (27), we have:

$$\frac{d}{ds} \int_0^\infty \frac{r^2 J_i(\xi r) \, dr}{\sqrt{s^2 - r^2}} = s \sin(\xi s).$$ (36)

$$\frac{d}{ds} \int_0^\infty \frac{J_i(\xi r) \, dr}{\sqrt{s^2 - r^2}} = \frac{s}{\xi} \left[ \frac{\cos(\xi s)}{s} - \frac{\sin(\xi s)}{s^2} \right].$$ (37)

Eqs. (28) and (29) are the Abel type integral equations, whose solutions are as follows:
\[(1 - \gamma_1 \gamma_2) F_2(s) + \frac{2}{\pi} \int_0^a \left\{ \frac{s}{s^2 - u^2} + \frac{1}{2u} \ln \left| \frac{s + u}{s - u} \right| \right\} du \]

\[F_1(u) du = \frac{2}{\pi \gamma_1 \gamma_2} \int_0^a \frac{F_1(u)}{s^2 - u^2} du + \frac{\gamma_1 \gamma_2 \pi}{2} \int_0^\infty \frac{F_2(u)}{u^2} du + \frac{\gamma_1 \gamma_2}{\pi} \int_0^a \frac{F_1(u)}{u} \ln \left| \frac{s - u}{s + u} \right| du + \frac{\gamma_1 \gamma_2}{\pi} \int_0^a \frac{F_1(u)}{u} \ln \left| \frac{s - u}{s + u} \right| du = \gamma_1 a \delta, \quad b < s < \infty. \tag{39}\]

We can rewrite Eqs. (38) and (39) in the general forms:

\[\phi_1(s) + \int_0^a \phi_1(u) K_{11}(u, s) du + \int_b^a \phi_2(u) K_{12}(u, s) du = L_1(s) \delta, \quad 0 < s < a, \tag{40}\]

\[R \phi_2(s) + \int_0^a \phi_1(u) K_{21}(u, s) du + \int_b^a \phi_2(u) K_{22}(u, s) du = L_2(s) \delta, \quad b < s < \infty, \tag{41}\]

where we have assumed that \(F_1(s)\) and \(F_2(s)\) admit representations of the form:

\[\left[ F_1(u); F_2(u) \right] = \frac{\gamma_1 u}{\pi} \left[ \phi_1(u); \phi_2(u) \right], \tag{42}\]

and kernel functions \(K_{ij} (i, j = 1, 2)\) are expressed as follows:

\[K_{11}(u, s) = \frac{\eta_1}{u^2 - s^2} \left( u \ln \left| \frac{a - s}{a + s} \right| - s \ln \left| \frac{a - u}{a + u} \right| \right), \]

\[K_{22}(u, s) = \eta_2 \frac{a}{s u}, \]

\[K_{12}(u, s) = \eta_3 \left( -\frac{s}{u^2 - s^2} + \frac{1}{2u} \ln \left| \frac{s + u}{s - u} \right| \right) + \eta_4 \frac{1}{2u} \ln \left| \frac{a + s}{a - s} \right|, \]

\[K_{21}(u, s) = \eta_3 \left( -\frac{u}{u^2 - s^2} + \frac{1}{2u} \ln \left| \frac{a + s}{a - s} \right| \right) + \eta_4 \frac{1}{2u} \ln \left| \frac{a + u}{a - u} \right|. \tag{43}\]

Constants \(R\) and \(\eta_k (k = 1, ..., 4)\) are given by:

\[R = 1 - \gamma_1 \gamma_2, \]

\[\eta_1 = -\frac{2}{\pi^2} \gamma_1 \gamma_2, \quad \eta_2 = \gamma_1 \gamma_2, \]

\[\eta_3 = \frac{2}{\pi} (1 - \gamma_1 \gamma_2), \quad \eta_4 = \frac{2}{\pi} \gamma_1 \gamma_2, \tag{44}\]

and functions \(L_1(s)\) and \(L_2(s)\) are defined by:

\[L_1(s) = \ln \left| \frac{s + a}{s - a} \right|, \quad L_2(s) = \frac{\pi a}{s}. \tag{45}\]

3. Contact-load distribution and axial stiffness

A practical interesting result is the load-displacement relationship. The resultant force, \(T\), acting on the disc is calculated by:

\[T = 2 \int_0^\pi \int_0^\infty r \sigma_{zz}({r, \theta}) dr d\theta, \tag{46}\]

where the axial stress, \(\sigma_{zz}\), in the inclusion region is given by:

\[\sigma_{zz}(r, \theta) = \int_0^\infty \xi M(\xi) J_0(\xi r) d\xi - \gamma_2 \int_0^\infty \xi N(\xi) J_0(\xi r) d\xi, \quad 0 < r \leq a. \tag{47}\]

By employing the identities \(M(\xi)\) and \(N(\xi)\) as defined in Eqs. (16) and (22), Eq. (46) simplifies to:

\[T = 8 \pi \eta \gamma \frac{\gamma_1 \eta_1}{\pi^2} \left[ \int_0^a \phi_1(u) \ln \left| \frac{a + u}{a - u} \right| du + \pi a \int_b^a \phi_2(u) \frac{u}{u} du \right]. \tag{48}\]

4. Stress intensity factor at the crack tip

A quantity of physical interest, which is applicable in fracture mechanics, is the stress intensity factor. Due to asymmetric deformation about \(z = 0\), the only non-zero stress component is \(\sigma_{\tau z}\). The mode II stress intensity factor is defined by:

\[K_{II}^b = \lim_{r \to a^+} [2(r - b)]^{1/2} \sigma_{\tau z}(r, 0), \tag{49}\]

where:

\[\sigma_{\tau z}(r, 0) = \int_0^\infty \xi N(\xi) J_1(\xi r) d\xi, \quad r > b. \tag{50}\]

Utilizing Eq. (21), we find that:

\[\sigma_{\tau z}(r, 0) = \frac{2 F_2(b)}{\pi r (r^2 - b^2)^{1/2}} + \frac{2}{\pi r} \int_0^r \frac{F_2(s) ds}{(r^2 - s^2)^{1/2}}. \tag{51}\]

Inserting Eqs. (42) and (51) into Eq. (49), the mode II stress intensity factor can be expressed as follows:
\[
K_{II}^b = \frac{2\gamma_1 c_2(b)}{\pi^2 \sqrt{b}}
\]

(52)

5. Special cases

Before proceeding to the numerical solution of the general problem, it is relevant to examine some limiting cases whose solutions are available. Herein, five special cases are inferred: (i) intact medium; (ii) completely cracked solid; (iii) direct loading of a penny-shaped crack; (iv) \( s_2 \rightarrow s_1 \); and (v) effect of incompressible materials.

5.1. Inclusion in an uncracked elastic solid full-space

The force \( T \) required to achieve the disc displacement \( \delta \) in \( z \) direction is equal to the following (see Figure 2(a)): \[
T = \frac{8c_{44}c_{330}a\delta(s_1 + s_2)}{c_{44} + \sqrt{c_{11}c_{33}}},
\]

(53)

where a transversely isotropic medium \([33]\) and an isotropic medium simplifies to \([35]\) are simplified to:

\[
T = \frac{32G a \delta (1 - \nu)}{3 - 4 \nu},
\]

(54)

in which \( G \) and \( \nu \) are the elastic shear modulus and Poisson’s ratio for an isotropic medium, respectively.

5.2. Disc inclusion embedded between two half-space regions

Considering the limit \( a/b \rightarrow 0 \) (with \( a \neq 0 \)), the problem turns into the disc inclusion which is embedded between two identical half-space regions (see Figure 2(b)). In this case, the total force is:

\[
T = \frac{2a a \delta}{H \tanh(\pi \theta)},
\]

(55)

where \( H \) and \( \theta \) are mentioned in Appendix B. This relation, corresponding to a transversely isotropic medium, as reported by Fabrikant \([35]\) and for an isotropic medium, simplifies to \([36]\):

\[
T = \frac{8Ga \delta \ln(3 - 4 \nu)}{1 - 2 \nu}.
\]

(56)

5.3. Direct loading of a penny-shaped crack

The exact mode II stress intensity factor of a penny-shaped crack in an infinite transversely isotropic medium due to a axial point force parallel to the \( z \)-axis applied at the center of the crack is obtained as follows \([37]\):

\[
K_{II}^b = \frac{T}{2\pi \sqrt{b}} \left( \frac{m_1 s_1}{m_1 - 1} + \frac{m_2 s_2}{m_2 - 1} \right),
\]

(57)

where \( m_k \) \((k = 1, 2)\) is defined in Appendix B, which, for the isotropic case, simplifies to \([8]\):

\[
K_{II}^b = \frac{T(1 - \nu)}{8\pi (1 - \nu) \sqrt{b}}.
\]

(58)

In this study, as \( a \rightarrow 0 \) (see Figure 2(c)), implying the inclusion disappears, parameter \( T \) in Eqs. (57) and (58) can be obtained from Eqs. (55) and (56), respectively.

5.4. \( s_1 \) and \( s_2 \) become equal

By substituting \( s_1 = s_2 \) into Eq. (6), terms with forms of 0/0 will be encountered. This occurs in transversely isotropic materials when \( \sqrt{c_{11}c_{33}} - c_{33} - 2c_{44} = 0 \).

In this case, one can obtain \( s_1 = s_2 = (c_{11}/c_{33})^{1/4} \). Therefore, in order to obtain displacement and stress potential relations of the case of \( s_1 = s_2 \), it is required to take the limits by setting \( s_2 \rightarrow s_1 \). The results are presented in Appendix C.

5.5. Material incompressibility effect in an isotropic medium

In the limiting case of material incompressibility \((\nu = 0.5)\), Eqs. (54) and (56) reduce to the same result as follows:

\[
T = 16Ga \delta.
\]

(59)
From the above relation, it is evident that in the incompressible elastic materials, the extent of cracking and an increase in the value of \( b \), on the plane containing the rigid inclusion, have no effect on the axial stiffness of the inclusion (see [38]).

6. The numerical evaluation of the governing integral equations

The simultaneous coupled Fredholm integral equations of the second kind (40) and (41) governing the axi-symmetric interaction of a penny-shaped crack and a rigid disc inclusion are not amenable to solution in an exact form. A variety techniques was proposed for the numerical solution of coupled systems of Fredholm integral equations of the general type described by Baker [30] and Atkinson [40].

To solve these coupled integral equations numerically, integration intervals \((0, a)\) and \((b, \infty)\) are divided into \( N_1 \), \( N_2 \) segments, respectively, and end points of the segments can be expressed as follows:

\[
x_i = (2i - 1) h_1 \quad \text{with} \quad i = 1, 2, \ldots, N_1, \]

\[
t_i = t_{i-1} + \alpha (t_{i-1} + t_{i+1}), \]

where \( h_1 = a/2N_1 \), \( t_1 = b \), \( t_2 = b + a/N_1 \), and \( \alpha \) is a constant of proportionality such that the interval \((b, \infty)\) is approximated in the numerical scheme. For the treatment of coupled integral equations (Eqs. (40) and (41)), according to standard quadrature method, the integral equations can be written in the discretized form as:

\[
\begin{bmatrix}
D_1 & D_2 \\
D_3 & D_4
\end{bmatrix}
\begin{bmatrix}
\phi_1(s) \\
\phi_2(s)
\end{bmatrix} =
\begin{bmatrix}
L_1(s) \\
L_2(s)
\end{bmatrix},
\]

(61)

where:

\[
D_1 = \delta_{ij} + \sum_{i=1}^{N_1} \sum_{j=1}^{N_1} \frac{w_j \eta_i}{u_j - a^2} \left[ u_j \ln \frac{a - s_i}{a + s_i} - s_i \ln \left| \frac{a - u_j}{a + u_j} \right| \right], \quad i \neq j,
\]

(62)

\[
D_2 = \delta_{ii} + \sum_{i=1}^{N_1} \frac{w_i \eta_i}{2s_i} \left[ \frac{2as_i}{a^2 - s_i^2} + \ln \frac{a - s_i}{a + s_i} \right], \quad i = j,
\]

(63)

\[
D_3 = \sum_{i=1}^{N_1} \sum_{j=1}^{N_1} w_j \eta_i \left[ -\frac{u_j}{u_j^2 - s_i^2} + \frac{1}{2s_i} \ln \left| \frac{s_i + u_j}{s_i - u_j} \right| \right] + \frac{1}{2a_j} \ln \left| \frac{a + s_i}{a - s_i} \right|,
\]

(64)

\[
D_4 = \sum_{i=1}^{N_1} \sum_{j=1}^{N_1} w_j \eta_i \frac{a}{s_i u_j},
\]

(65)

\[
\delta_{ij} = \begin{cases}
\frac{a}{N_1}, & 0 < r < a \\
\frac{t_{i+1} - t_i}{b - r < \infty}
\end{cases}
\]

(67)

The total load acting on the inclusion from Eq. (48) is:

\[
\frac{T}{\delta} = 8a \eta_k + \frac{8 \gamma_1 \eta_k}{\pi^2} \int_{-1}^{1} w_i \phi_1(u_i) \ln \left| \frac{a + u_i}{a - u_i} \right| \\
+ \frac{\pi a}{\pi^2} \sum_{i=1}^{N_1} \frac{w_i \phi_2(u_i)}{u_i},
\]

(68)

It can be written in terms of axial stiffness which is defined as follows:

\[
\tilde{T} = \frac{T}{\epsilon_{44} \alpha \delta}.
\]

(69)

The stress intensity factor, defined by Eq. (52), can be expressed in the form of:

\[
K_{II}^{b} = \frac{2 \delta \gamma_1 \phi_2(b)}{\pi^2 \sqrt{b}}.
\]

(70)

The normalized stress intensity factor can take the following form:

\[
\tilde{K}_{II}^{b} = \frac{K_{II}^{b} \delta^{1/2}}{\epsilon_{44} \alpha \delta}.
\]

(71)

7. Numerical results and discussion

To confirm the validity of the present solution and evaluate the effects of anisotropic materials on the results, several synthetic types of isotropic (material 1) and transversely isotropic materials (materials 2-9) are selected. The material properties are given in Table 1, where \( E \) and \( E' \) are the Young’s modulus on the plane of isotropy and perpendicular to it, respectively; \( \nu' \) is Poisson’s ratio that characterizes the effect of horizontal strain on the complementary vertical strain; \( \nu \) is the Poisson ratio which characterizes the effect of
vertical strain on the horizontal one; and $G'$ stands for the shear modulus on the plane normal to the plane of isotropy. Regarding the positive-definiteness of strain energy, the following constraints for material constants $c_{ij}$ have been checked (for example, see [41])

$$c_{11} > |c_{12}|, \quad (c_{11} + c_{12}) c_{33} > 2 c_{13}^2, \quad c_{44} > 0.$$ (72)

Axial stiffness as a function of the inclusion-crack aspect ratio is plotted in Figure 3. The influence of $E' / E$ ratio is shown in Figure 3(a), implying that an increase in $E' / E$ leads to a remarkable increase in the axial stiffness. From Figure 3(b), one might notice that by decreasing $G' / G$, the stiffness increases significantly. It can be concluded that anisotropic parameters $E'$ and $G'$ have the major influence on the axial stiffness. In contrast, from Figure 3(c), one can observe that the increase of $E / E'$ and $G / G'$ has little effect on the results. However, anisotropic parameters $E$ and $G$ are found to be of minor importance for the axial interaction of crack-inclusion.

The results of the synthetic transversely isotropic materials for the normalized shearing stress intensity factor are presented in Figure 4. As indicated in Figure 4(a), the larger the value of $E' / E$ is, the higher the response rate will be. Figure 4(b) shows that the reduction of $G' / G$ leads to a slight increase in the mode II stress intensity factor. As shown in Figure 4(c), changing the ratios of $E / E'$ and $G / G'$ has a significant influence on $\sigma_{zz}$ and $K_{zz}^2$.

In the limiting $a \rightarrow b$, the normalized stress intensity factor decreases sharply, which is due to the oscillatory stress singularity. Selvadurai [12] showed that such oscillatory stress singularities have virtually no influence on the accuracy of the translational stiffness. In this study, $\tilde{K}_{zz}^2$ is depicted for the interval (0.0.9). Hilbert solution has been suggested to overcome this problem in the integral transform method.

8. Conclusions

The analytical treatment of the interaction crack-inclusion in a transversely isotropic full-space was revisited. By virtue of appropriate potential functions, the mixed boundary value problem was reduced to dual and triple integral equations. By employing suitable representations and Abel transforms, the results were expressed in terms of the solution of two coupled Fred-
holm integral equations, solved by using a numerical method. The available closed-form results derived from the literature, such as the inclusion in an intact medium, were recovered as the limiting cases of the current study. The axial stiffness of the inclusion and the shearing stress intensity factor at the tip of the crack were obtained for some synthetic transversely isotropic materials. The effects of material anisotropy on the results were also highlighted.

References


Appendix A

The parameters in Eqs. (10)-(13) are:

\[ \gamma_1 = -\frac{C_2}{C_1}, \]  
\[ \gamma_2 = -\frac{C_3}{C_4}, \]  
\[ C_1 = -\frac{1 + \alpha_1 + s_1 s_2 \alpha_2}{c_{44} (s_1 + s_2) (1 + \alpha_1)}, \]  
\[ C_2 = -\frac{1 - \alpha_1 + s_1 s_2 (\alpha_3 - \alpha_2)}{(s_1 + s_2) (1 + \alpha_1)}. \]

Eqs. (A.5) to (A.8) are shown in Box A.I.

Appendix B

The parameters in Eqs. (55) and (57) are:

\[ m_k = \frac{c_{44} + c_{44}}{c_{33} s_k^2 - c_{44}} \quad (k = 1, 2). \]  
\[ \theta = \frac{1}{2\pi} \ln \left[ \sqrt{\gamma_1^2 + \alpha} \right], \]  
\[ H = \frac{(\gamma_1^2 + \gamma_2^2) c_{11}}{2\pi (c_{11} c_{33} - c_{13}^2)}, \]  
\[ \alpha = \frac{\sqrt{c_{11} c_{33} - c_{13}^2}}{c_{11} (\gamma_1^2 + \gamma_2^2)}. \]  
\[ \gamma_k = s_k^{-1} = \frac{m_k c_{44}}{m_k c_{44} + c_{13} + c_{44}} \quad (k = 1, 2). \]
\[
C_3 = \frac{c_{20} \left(1 + \alpha_1 - s_1^2 \alpha_2 \right) \left(1 + \alpha_1 - s_2^2 \alpha_2 \right) - c_{13} \left(1 + \alpha_1 + s_1 s_2 \alpha_2 \right) \alpha_3}{c_{44} \left(1 + \alpha_1 \right) \alpha_3}, \quad (A.5)
\]

\[
C_4 = \frac{c_{20} \left(1 + \alpha_1 - s_1^2 \alpha_2 \right) \left(1 + \alpha_1 - s_2^2 \alpha_2 \right) - c_{33} \left(1 + \alpha_1 + s_1 s_2 \alpha_2 \right) \alpha_3 + c_{13} s_1 s_2 \alpha_3^2}{(s_1 + s_2) \left(1 + \alpha_1 \right) \alpha_3}, \quad (A.6)
\]

\[
A(\xi) = \frac{(N(\xi) + M(\xi) c_{44}) \left(1 + \alpha_1 - s_1^2 \alpha_2 \right) + M(\xi) c_{44} s_1 \alpha_3}{\xi c_{44} \left(s_1^2 - s_2^2 \right) \left(1 + \alpha_1 \right) \alpha_3}, \quad (A.7)
\]

\[
B(\xi) = \frac{(N(\xi) + M(\xi) c_{44}) \left(1 + \alpha_1 - s_1^2 \alpha_2 \right) + M(\xi) c_{44} s_1 \alpha_3}{\xi c_{44} \left(s_1^2 - s_2^2 \right) \left(1 + \alpha_1 \right) \alpha_3}, \quad (A.8)
\]

Box A.1

\[
u_z(r, z) = \int_0^\infty \frac{\xi z k_1 (N(\xi) + M(\xi) c_{44}) + M(\xi) \omega_1(\xi) c_{44} s_1 \alpha_3 e^{-\xi s_1 z} J_0(\xi r) d\xi}{2 c_{44} s_1 \left(1 + \alpha_1 \right) \alpha_3}, \quad (C.1)
\]

\[
u_r(r, z) = \int_0^\infty \frac{(N(\xi) + M(\xi) c_{44}) \omega_2(\xi) + M(\xi) c_{44} s_1 \alpha_3 \left(1 + \xi s_1 z \right) e^{-\xi s_1 z} J_1(\xi r) d\xi}{2 c_{44} s_1 \left(1 + \alpha_1 \right) \alpha_3}, \quad (C.2)
\]

\[
\sigma_{zz}(r, z) = -\int_0^\infty \frac{c_{13} \alpha_3 \Gamma_1(\xi) + c_{33} \Gamma_2(\xi) \xi e^{-\xi s_1 z} d\xi}{2 c_{44} s_1 \left(1 + \alpha_1 \right) \alpha_3}, \quad (C.3)
\]

\[
\sigma_{zz}(r, z) = -\int_0^\infty \frac{\xi z k_1 (N(\xi) + \alpha_3 s_1 \omega_3(\xi) N(\xi) + c_{44} \xi s_2 M(\xi) \xi e^{-\xi s_1 z} J_1(\xi r) d\xi}{2 s_1 \left(1 + \alpha_1 \right) \alpha_3}, \quad (C.4)
\]

Box C.1

**Appendix C**

Displacement and stress potential relations of the case of \(s_1 = s_2\) are shown in Box C.1, where:

\[
\kappa_1 = \left(1 + \alpha_1 - s_1^2 \alpha_2 \right)^2, \quad (C.5)
\]

\[
\kappa_2 = \left(1 + \alpha_1 + s_1^2 \left(\alpha_3 - \alpha_2 \right) \right)^2, \quad (C.6)
\]

\[
\omega_1(\xi) = \left(2 + \xi s_1 z \right) \left(1 + \alpha_1 \right) - \xi s_1^2 \alpha_2 z, \quad (C.7)
\]

\[
\omega_2(\xi) = \left(\xi s_1 z - 1 \right) \left(1 + \alpha_1 \right) - s_1^2 \alpha_2 + \left(1 + \xi s_1 z \right), \quad (C.8)
\]

\[
\omega_3(\xi) = \left(\xi s_1 z - 2 \right) \left(1 + \alpha_1 \right) - \xi s_1^2 \alpha_2 z, \quad (C.9)
\]

\[
\omega_4(\xi) = 1 + \xi s_1 z, \quad (C.10)
\]

\[
\omega_5(\xi) = 1 - \xi s_1 z, \quad (C.11)
\]

\[
\Gamma_1(\xi) = \omega_3(\xi) \left(N(\xi) + M(\xi) c_{44} \left(1 + \alpha_1 \right) \right) + M(\xi) c_{44} s_1 \alpha_3 \left(\omega_4(\xi) + \xi s_1^2 \alpha_2 \right), \quad (C.12)
\]

\[
\Gamma_2(\xi) = -\omega_5(\xi) \kappa_1 \left(N(\xi) + M(\xi) c_{44} \right) + M(\xi) c_{44} s_1 \alpha_3 \left(\omega_4(\xi) + \xi s_1^2 \alpha_2 \right), \quad (C.13)
\]

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