Water flow stabilization using submerged weir for draft-tube reaction hydraulic turbine

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Hydropower sustainability;
Water pressure and velocity;
Submerged weir;
Reaction turbine.

Abstract. In turbine engineering, draft-tube downstream runs under extreme water flow pressure and velocity, causing vibration and pressure variation in different operation frequencies. The practical challenge of obtaining stabilized water flow is an ongoing domain of research. This paper investigates a proposition of applying a submerged weir at the downstream end of the draft-tube reaction turbine. The main goal of this research is to reduce variations in water flow pressure, velocity, and shear distribution in accordance with the effect of the upstream water level. Two types of turbines, including vertical Kaplan and Francis turbine units, are examined. ANSYS CFX software tool is used to build three-dimensionally (3D) numerical models for the Kaplan and Francis turbines by building a submerged weir at the outlet of the draft tubes based on three different height suggestions. The effect of the proposed submerged weir on the flow through these turbines is studied by considering the dimensions of their components including the penstock with inlets, spiral casing, shafts and blades, and the draft tube with outlets. The findings of this research are significant enough to solve the problem of negative pressure pulsation in the draft tube of Kaplan and Francis turbines types.

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1. Introduction

Powerhouses are among the main parts of dams that are used to generate low-cost hydroelectric power. The hydraulic characteristics of Kaplan and Francis turbine units [1] are identified that function as the main engine of a powerhouse. Kaplan and Francis turbines, which are classified as reaction turbines, are difficult to use under part-load operation because of pressure oscillation [2–4]. Studies on this topic have presented solutions for the cavitation problem in draft tubes [5], the vibration effect in powerhouses caused by a running turbine, maximization of power generation, and generation of low-cost power [1,6].

Over the last decade, there has been a very noticeable development in the computational fluid tools [7–9]. It has become very effective in performing a robust and reliable analysis of the flow pattern phenomena inside the turbine structure. Based on the literature, numerous studies have been conducted by utilizing the aforementioned tools to simulate the flow behavior in the draft tube of a turbine and inspect the critical condition such as vortex rope and vibration [2,10]. Researchers have studied pressure pulsation in Francis hydraulic turbine units and discussed the cavitation phenomenon problem [11,12]. Jošt and Lipej (2011)
built a 3D numerical model for a Francis turbine unit to predict vortex rope in the draft tube based on numerical flow analyses [13]. Two analyses with and without cavitation effects were performed. Another study performed a numerical analysis of cavitation turbulent flow in a Francis turbine under partial load operation using the $k - \omega$ shear stress transport turbulence model in Reynolds-averaged Navier-Stokes equations [14]. Qian et al. (2007) simulated 3D multiphase flow in a Francis turbine to calculate pressure pulsation in the spiral casing, draft tube, runner front, and guide vanes using fast Fourier transform [15]. The investigation of the hydrodynamic effects of pressure fluctuation in the draft tube was studied [16]. The cause of the simulated rotor-stator interaction under partial load operation was investigated by analyzing the 3D transient-state turbulence flow simulation of a Francis turbine. The 3D Navier-Stokes Computational Fluid Dynamics (CFD) solver ANSYS CFX was used to analyze flow through a vertical Francis turbine with different loads in situ. Lately, Luna-Ramírez et al. (2016) calculated pressure on the blades of a 200 MW Francis hydraulic turbine units and presented methods to reduce the cavitation problem. Ko and Kurosawa (2014) evaluated and presented cavitation performance at a specific speed for a 400 MW Kaplan turbine using a finite volume method to solve the Reynolds-averaged Navier-Stokes equations combined with the Reynolds stress model [24]. The modified Rayleigh-Plesset equation was used to model the collapse and growth of cavitation bubbles. Javadi and Nilsson (2014) adopted the renormalization group $k - \varepsilon$ turbulence model combined with the Reynolds-averaged Navier-Stokes equations to analyze unsteady turbulent flow in a U9 Kaplan turbine model [25]. Analyses were performed on the fluctuation of pressure in the draft tube, unsteady flow behavior, and cohesive flow structures. Another investigation was carried out on the runner outlet flow of a Francis turbine model using a two-component particle image velocimetry system [26]. The finding of the research proposed a particular shape that provides suitable optical access across the draft tube elbow. The characteristics of the flow pattern in a Francis turbine runner with a small opening valve using the Reynolds-averaged Navier-Stokes equations and the continuity equation were inspected [27]. The 3D unsteady turbulence flow throughout the entire passage of the turbine was simulated numerically based on the $k - \varepsilon$ two-equation turbulence model using the CFD software ANSYS Fluent. The finding of the study showed that a low-pressure zone expanded around the blades of the runner when the valve was closed, and velocity increased throughout the runner area. On the other hand, the effect of hydraulic instabilities on increasing the service lifetime of Francis and Kaplan turbines was accomplished [1]. In particular, Caihui (2012) built a mathematical model to study pressure distribution in the flow pattern inside the powerhouse of a hydropower station using a fluid dynamics method (CFD) to determine the velocity distribution and pressure pattern distribution under three operating conditions: one-unit load, two-unit load, and full-load rejection [28]. The results of this study outlined good flow patterns at the inlet with steady water level fluctuations. Many state-of-the-art studies on the pressure pulsation in the draft tube of Kaplan and Francis turbines rely on the same methods and analyses, yet use different models. Based on a comprehensive review of the article published [2], several researchers have suggested changes in turbine design to reduce the cavitation phenomenon and increase turbine efficiency.

In the current work and to the best knowledge of the authors, construction of a submerged weir with three different heights (i.e., 1/6, 1/3, and 1/2 from the draft-tube outlet height) was suggested to decrease the pressure variation in the draft tubes of turbine units. The main enthusiasm for establishing this research results from the main concept of broad-crested weir that stabilizes the flow through the open channel [29,30]. Two reaction hydraulic turbines in two different embankment dams are selected as a real practical example to evaluate hydraulic performance. The hydraulic performances include pressure stability in the draft tubes of a vertical Kaplan turbine used in the Haditha Power Station and a vertical Francis turbine used in the Temenggor Power Station. A 3D numerical model with one turbine unit for each powerhouse is analyzed and simulated using ANSYS CFX software tool at different water levels (minimum to maximum). The discharge and hydraulic information was obtained from sites. Flow velocity, pressure distribution, and shear wall distribution were determined in different loading cases using the $k - \varepsilon$ turbulence model. The finite volume method was adopted, and the physical properties and flow characteristics of water were defined. The simulation results of the models determined the characteristics of the turbines obtained by running the 3D turbine models, which are changeable with respect to the upstream and downstream water levels and discharge ranges. This study provides a foundation for determining the hydraulic characteristic performance of reaction turbines to compare the two types of hydropower station: The Haditha powerhouse, which is an integral part of the dam body, and the
Temenggor powerhouse, which is separate from the dam body. A safe and low-cost method for generating hydroelectric power can be identified.

2. Dams and power stations description

2.1. Haditha Dam
The Haditha Dam is an earth-fill dam located on the Euphrates River, north of Haditha City, Iraq. This dam is over 9 km long and 57 m high. The Haditha Dam was built to generate hydroelectricity and regulate water for irrigation [31]. The power station in the Haditha Dam contains 6 vertical Kaplan turbines that can generate 600 MW of electricity. Figure 1(a) outlines the downstream flow of the Haditha Dam with 6 outlets opening into the spillway, 12 outlets opening into the power station, and 2 outlets opening into each turbine unit. Full details of the hydraulic characteristics of the dam are tabulated in Table 1.

2.2. Temenggor Dam
The Temenggor Dam is the third largest dam in Malaysia, and the Temenggor Power Station is one of the largest hydroelectric power generation facilities in Malaysia. It is located in Sungai Perak, approximately 200 km northeast of Ipoh state [32]. This rock-fill dam has a height of 128 m and a crest length of 537 m. The Temenggor Power Station has four vertical Francis turbines with an installed capacity of 348 MW; it is considered a separate powerhouse. Figure 1(b) illustrates the Temenggor Dam with eight outlets of the surface downstream power station. Table 1 presents the hydraulic information required to build the models for the turbine units.

3. Methodology overview
This reaction turbine represents one of the largest hydraulic structures. Water pressure applies force to runner blades, and this pressure decreases throughout the running turbine; this phenomenon was observed in both Francis and Kaplan turbines. The runner and the blades of the turbines are fully immersed in water and must be sufficiently strong to resist the operating

| Table 1. The hydraulic information of Haditha and Temenggor power stations. |
|---------------------------------|----------------|----------------|
| Properties                      | Haditha Dam    | Temenggor Dam  |
| Type of the dam                 | Earthfill      | Rockfill       |
| Location                        | 34°12′25″N, 42°21′18″E | 5°21′24″N, 101°18′4″E |
| Power house type                | Integral part of dam body | Surface power house |
| Type of turbines                | Vertical Kaplan | Vertical Francis |
| Number of units                 | 6              | 4              |
| Install capacity (MW)           | 6 x 110 = 600  | 4 x 87 = 348   |
| Length of unit (m)              | 67.35          | 260            |
| Penstock diameter (m)           | The details shown in Figure 2 | 5.5 |
| Maximum U/S.W. L (m)            | 130.2          | 248.42         |
| Minimum U/S.W. L (m)            | 129            | 236.5          |
| D/S.W. L (m)                    | 107.3          | 142            |
| Maximum power house discharge (m³/s) | 6 x 339 = 2034 | 4 x 100 = 400 |
| Spillway                        | Included dam body | Separated structure |
| Max. spillway discharge (m³/s)  | 11000          | 2883           |
pressure. The hydraulic power of the turbine units is
given in Eq. (1) [33,34]:

\[ P = \rho Q g H \eta, \]  

(1)

where \( P \) is the water pressure, \( \rho \) is the water mass
density, \( Q \) is the water discharge, \( g \) is the gravity
weight, \( H \) is the water head, and \( \eta \) is the efficiency.

The energy of turbines \( E \) is defined as can be seen
in Eq. (2) [35]:

\[ E = g H_1 = \left( \frac{p_1 - p_2}{\rho} \right) + \left( \frac{V_1^2 - V_2^2}{2} \right) + g (z_1 - z_2) + \text{head loss}_{1-2}, \]  

(2)

where \( H_1 \) is the water head of the turbine. Sections 1
and 2 are defined as the upstream and downstream
measurements of the turbine, respectively. The
determined behavior of the hydraulic turbine models
is based on a dimensional analysis. Laboratory
developments and model tests can guarantee hydraulic
behavior and turbine efficiency [36]. The
International Electrotechnical Commission Standards 60193
and 60041 define all the simulation rules [37,38]. The
specific speed of a turbine based on these standards is
defined in the following formula [34,39]:

\[ n_{QE} = \frac{n \sqrt{Q}}{(gH_n)^{1/4}}. \]  

(3)

Parameter \( n_{QE} \) is known as the specific speed of
any type of turbines. Numerous statistical studies
on reaction turbines have established a correlation
between speed and net head for each type of turbine.
The Schweiger and Gregory correlation formulas for
Kaplan turbines are defined as follows [40]:

Kaplan \( n_{QE} = \frac{2.294}{H_n^{1.26}} \)  

(4)

The Lugaserti and Mass correlation formulas for Francis
turbines are presented as follows [41]:

Francis \( n_{QE} = \frac{1.924}{H_n^{5.12}} \)  

(5)

The previous statistical formulas are used only for
preliminary studies during the first trial to estimate the
specific speed so as to set the rotational speed of
a turbine by applying Eqs. (3), (4), and (5), because
there exists no clear relationship among the head, flow
rate, and the rotational speed of the turbine.

4. 3D numerical finite element turbine model

In this study, two different kinds of turbines (e.g.,
Kaplan and Francis) are selected as case studies to
investigate. ANSYS CFX is used to simulate the 3D
numerical finite-volume flow turbine models including
the runner with blades, and the shaft is defined as the
submerged rotational body. The water field includes
two rectangular inlets for the Kaplan turbine and one
round inlet for the Francis turbine. The penstock,
the spiral casing, the draft tube, and two rectangular
outlets are defined. The boundary conditions include
the discharge range, the operating head, the rotational
speed of the turbines, and the effect of gravity. The
Two models run with three submerged weirs are recom-
medated to be applied to the downstream of turbine
units. Figure 2(a) and (b) display the dimensions of
the units.

The ANSYS-CFX software used for simulation
relies on the finite-volume method. The first step of
the calculation is study-state flow field, and the result
of this step represents the initial condition for the next
step. The flow simulation of the Francis turbine unit
was employed by using several meshes to test grid
independence. After many iterations, the calculations
reach convergence.

The second step in turbine modeling involves the
selection of a suitable finite volume mesh. The grid of
the turbine is made of tetrahedral elements after per-
forming several trials to determine the smallest possible
aspect ratio under 150 and the minimum orthogonal
cover 0.15 in accordance with ANSYS-CFX code
recommendations, whereas analysis of hexahedral elements
was performed for the walls’ boundary layers. The final
mesh satisfies \( y^+ < 200 \) around the boundary wall to
obtain the required pressure fluctuation, following the
previous research conducted by [42]. The runner, guide
vanes, and draft-tube interactions were evaluated by
slip meshes. This mesh slipping was observed to be
interacting on the sides of interface. However, it is
important to ensure that velocity components, pressure,
and flow flux are harmonious after interpolation. The
meshing details used in the Haditha Kaplan turbine
model and Temenggor Francis turbine are shown in
Table 2. The number of elements and nodes used
in Temenggor turbine meshing is higher than that in
the Haditha turbine model, because the Temenggor
turbine unit is longer than the Haditha turbine unit.
The Francis runner with 12 blades has several fine
details represented by small elements, whereas the
Kaplan runner includes only 6 blades with details larger
than those of the Temenggor runner turbine. The
Outlet boundary condition, the relative static pressure,
turbulent kinetic energy, and its diffusion rate are
prescribed; no-slip boundary condition is applied to
the wall, and standard wall functions are applied to
the region near the wall [43]. Figure 3 illustrates the
examples of the meshing details for the two-component
turbine model.

The water flow through the draft tube was mod-
eled using the incompressible continuity formulation and Reynolds time average. The mathematical explanation can be presented as follows [42]. The water flow continuity formula is:

\[
\frac{\partial u_j}{\partial x_j} = 0. \tag{6}
\]

In addition, the momentum formula is presented as follows:

\[
\rho \frac{\partial u_i}{\partial t} + \rho u_j \frac{\partial u_i}{\partial x_j} = \rho F_i - \frac{\partial P}{\partial x_i} + \mu \frac{\partial^2 u_i}{\partial x_j \partial x_i} - \rho \frac{\partial(u_i' u_j')}{\partial x_j}, \tag{7}
\]

where:

\[
-\rho u_i' u_j' = \mu \left( \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right) - \frac{2}{3} \left( \mu k + \mu \frac{\partial u_i}{\partial x_i} \right) \dot{\epsilon}_{ij}, \tag{8}
\]

The double formula of the \( k - \varepsilon \) is:

\[
\rho \frac{Dk}{Dt} = \frac{\partial}{\partial x_j} \left( \alpha_k \mu_{ef} \frac{\partial k}{\partial x_j} \right) + 2 \mu_k \dot{\epsilon}_{ij} \frac{\partial u_i}{\partial x_j} + \rho \varepsilon, \tag{9}
\]

\[
\rho \frac{D\varepsilon}{Dt} = \frac{\partial}{\partial x_j} \left( \alpha_\varepsilon \mu_{ef} \frac{\partial \varepsilon}{\partial x_j} \right) + 2 C_{1\varepsilon} \frac{\varepsilon}{k} \dot{\varepsilon}_{ij} \frac{\partial u_i}{\partial x_j} - C_2 \varepsilon^2 - R, \tag{10}
\]

where \( \dot{\epsilon}_{ij} = \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \), \( \mu_{ef} = \mu_t + \mu \), and \( \mu_t = C_{\mu} \frac{k^2}{\varepsilon} \).

\( R \) can be determined by:

\[
R = \frac{C_{\mu} \rho \mu^3}{1 + \beta \mu^3} \frac{\varepsilon^2}{k}. \tag{11}
\]

**Figure 2.** (a) 3D model of a vertical Kaplan unit with dimensions. (b) 3D model of a vertical Francis unit with dimensions.
Table 2. The meshing details of Haditha Kaplan turbine unit and Temenggor Francis turbine unit.

<table>
<thead>
<tr>
<th>Mesh details</th>
<th>Nodes</th>
<th>Elements</th>
<th>Max. aspect ratio</th>
<th>Minimum orthogonal quality</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>Haditha Kaplan:</strong></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Without weir</td>
<td>Water</td>
<td>9785833</td>
<td>2174630</td>
<td>10.706</td>
</tr>
<tr>
<td></td>
<td>Turbine</td>
<td>1808916</td>
<td>401988</td>
<td>10.706</td>
</tr>
<tr>
<td>Weir height = 1.333 m</td>
<td>Water</td>
<td>10371281</td>
<td>2304729</td>
<td>11.216</td>
</tr>
<tr>
<td></td>
<td>Turbine</td>
<td>1835460</td>
<td>407880</td>
<td>11.216</td>
</tr>
<tr>
<td>Weir height = 2.667 m</td>
<td>Water</td>
<td>10480064</td>
<td>2328003</td>
<td>12.012</td>
</tr>
<tr>
<td></td>
<td>Turbine</td>
<td>1843668</td>
<td>409704</td>
<td>12.012</td>
</tr>
<tr>
<td>Weir height = 4 m</td>
<td>Water</td>
<td>105382812</td>
<td>2354063</td>
<td>12.214</td>
</tr>
<tr>
<td></td>
<td>Turbine</td>
<td>1844775</td>
<td>409950</td>
<td>12.214</td>
</tr>
<tr>
<td><strong>Temenggor Francis:</strong></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Without weir</td>
<td>Water</td>
<td>97242978</td>
<td>2160955</td>
<td>14.561</td>
</tr>
<tr>
<td></td>
<td>Turbine</td>
<td>2278165</td>
<td>506259</td>
<td>14.561</td>
</tr>
<tr>
<td>Weir height = 0.5 m</td>
<td>Water</td>
<td>10092938</td>
<td>2242875</td>
<td>14.263</td>
</tr>
<tr>
<td></td>
<td>Turbine</td>
<td>2276995</td>
<td>505909</td>
<td>14.263</td>
</tr>
<tr>
<td>Weir height = 1 m</td>
<td>Water</td>
<td>10099465</td>
<td>2243235</td>
<td>14.202</td>
</tr>
<tr>
<td></td>
<td>Turbine</td>
<td>2275416</td>
<td>505648</td>
<td>14.202</td>
</tr>
<tr>
<td>Weir height = 1.5 m</td>
<td>Water</td>
<td>10137060</td>
<td>2252680</td>
<td>13.94</td>
</tr>
<tr>
<td></td>
<td>Turbine</td>
<td>2277756</td>
<td>506168</td>
<td>13.94</td>
</tr>
</tbody>
</table>

where \( \eta = S k / \varepsilon \), \( \eta_b = 4.38 \), \( C_\mu = 0.0845 \), \( \beta = 0.012 \), \( C_{1_\varepsilon} = 1.42 \) "originally in the model procedure”, \( C_{2_\varepsilon} = 1.68 \), \( \alpha_k = 1.0 \), and \( \alpha_{\varepsilon} = 0.769 \). Among these constants used in the turbulence model, a properly chosen value of \( C_{1_\varepsilon} \) is essential for improving the prediction of the pressure variation. In the present simulation, \( C_{1_\varepsilon} = 1.45 \) was selected based on the preliminary computations.

5. Description of hydraulic simulation

It is essential to use the \( k - \varepsilon \) turbulence model to describe the motion of turbulent flow through the turbine unit and, particularly, due to streamlines, to exhibit random motion near the turbine runner \([25, 27, 44] \). The 3D numerical model based on the finite volume method is used to distinguish the unsteady incompressible flow inside the turbine unit running under the varying head and discharge ranges and to solve the Reynolds-averaged Navier-Stokes equations. All simulations were applied according to Computational Fluid Dynamic (CFD) approach. The hydraulic data (i.e., upstream and downstream water levels with discharge ranges) required to operate the model were collected from engineering reports that belong to the inspected case studies. Table 3 provides the hydraulic data of Haditha and Temenggor powerhouses, and specific speed (column 3) is calculated using Eqs. (4) and (5) during the first trial, respectively. Eq. (1) is used to calculate the hydraulic input power using the efficiency of the Haditha Kaplan and Temenggor Francis turbines with results of 71.1% and 83.4%, respectively.
**Figure 3.** (a) 3D model of a vertical Kaplan unit with mesh details. (b) FE-Meshing of Haditha turbine runner model. (c) 3D model of vertical Francis unit with mesh details. (d) FE-Meshing of the Temenggor turbine runner model.

**Table 3.** Hydraulic calculations for Haditha turbine and Temenggor turbine units.

<table>
<thead>
<tr>
<th>No.</th>
<th>U/S.W. L (m)</th>
<th>Net head (m)</th>
<th>(N_{GE} )</th>
<th>(Q ) (m³/s)</th>
<th>(P ) (KW)</th>
<th>(V_{inlet} ) (m/s)</th>
<th>(N ) (rad/s)</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
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<td></td>
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<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td><strong>Haditha turbine</strong></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>1</td>
<td>129</td>
<td>18.5</td>
<td>0.6779</td>
<td>100</td>
<td>25807</td>
<td>1.5038</td>
<td>3.3520</td>
</tr>
<tr>
<td>2</td>
<td>134.3</td>
<td>25.5</td>
<td>0.5800</td>
<td>118</td>
<td>41975</td>
<td>1.7744</td>
<td>3.3586</td>
</tr>
<tr>
<td>3</td>
<td>139.6</td>
<td>32.5</td>
<td>0.5155</td>
<td>136</td>
<td>61658</td>
<td>2.0451</td>
<td>3.3353</td>
</tr>
<tr>
<td>4</td>
<td>144.9</td>
<td>20.5</td>
<td>0.4689</td>
<td>151</td>
<td>83204</td>
<td>2.2707</td>
<td>3.3326</td>
</tr>
<tr>
<td>5</td>
<td>150.2</td>
<td>46.5</td>
<td>0.4331</td>
<td>160.5</td>
<td>109049</td>
<td>2.5489</td>
<td>3.2839</td>
</tr>
<tr>
<td><strong>Temenggor turbine</strong></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>1</td>
<td>236.5</td>
<td>91.50</td>
<td>0.2058</td>
<td>50.0</td>
<td>38658</td>
<td>2.1045</td>
<td>4.2671</td>
</tr>
<tr>
<td>2</td>
<td>239.48</td>
<td>97.48</td>
<td>0.2068</td>
<td>65.0</td>
<td>51840</td>
<td>2.7359</td>
<td>3.8491</td>
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<tr>
<td>3</td>
<td>242.46</td>
<td>100.46</td>
<td>0.2036</td>
<td>75.0</td>
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<td>3.1568</td>
<td>3.6091</td>
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<td>4</td>
<td>245.44</td>
<td>103.44</td>
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<td>5</td>
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<td>106.42</td>
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<td>50.0</td>
<td>38658</td>
<td>2.1045</td>
<td>4.2671</td>
</tr>
</tbody>
</table>

The inlet velocity calculated by applying the continuity equation and rotational speed are verified using Eq. (3). The Kaplan and Francis numerical 3D models were run by defining the gravity weight of water and the turbine components including the runner and the shaft as the submerged rotational bodies. The boundary conditions include inlet velocity (column 7 in Table 3), outlet pressure (1 atm), and the rotational speed of the turbine (column 8 in Table 3). The central axis turbine is defined as the rotational
Table 4. Total head and percentage of error calculation at the inlets of the Haditha turbine unit and the Temenggor turbine unit.

<table>
<thead>
<tr>
<th>Haditha turbine</th>
<th>Temenggor turbine</th>
</tr>
</thead>
<tbody>
<tr>
<td>No.</td>
<td>U/S.W. L (m)</td>
</tr>
<tr>
<td>1</td>
<td>129</td>
</tr>
<tr>
<td>2</td>
<td>134.3</td>
</tr>
<tr>
<td>3</td>
<td>139.6</td>
</tr>
<tr>
<td>4</td>
<td>144.9</td>
</tr>
<tr>
<td>5</td>
<td>150.2</td>
</tr>
</tbody>
</table>

axis, and the time step interval is 500 steps/0.01 s to show the rotational motion of the turbine around its axis [45]. The turbine models were run several times by changing the rotational speeds of the turbine so that the pressure at the turbine inlets can be calculated; in doing so, the total head that is the summation of elevation head, velocity head, and pressure head, as tabulated in Table 4, is provided.

6. Application and findings analysis

As a matter of fact, turbines are usually designed locally in accordance with the dam conditions such as upstream and downstream water levels, water discharge, and various other hydraulic and hydrological factors. In addition, the efficiency of turbines is usually determined based on the off-design situation, especially in the case of the draft-tube turbine. Hence, there is a probability in its operation and reliability. Thus, it is necessary to maintain an optimal dimension of the individual components according to several properties (e.g., nature of flow and water level fluctuation). The motivation of this research is to optimize the draft-tube outlet by minimizing pressure fluctuation and fluent water velocity. Practically, two types of operating turbines, including Kaplan and Francis, were demonstrated as an example of the inspected application. According to Table 4, the calculation of the total head at the inlet of the Haditha and Temenggor turbine units is presented.

The phenomenon of fluid pattern is highly complex; thus, the advantage of Computational Fluid Dynamic (CFD) can be used to analyze fluid flow through the hydraulic mechanics concept. The CFD procedure is governed by dividing the fluid flow into finite volumes that can be solved numerically. In this work, the computational process was obtained by running the turbine models with different water levels, as shown in Table 3. The input rotational speed of a turbine is used, and several runs are performed to determine the inlet pressure that provides the total head at the inlet closest to the upstream water level.

In addition, Table 4 outlines the error percentage between the total head at the turbine inlet estimated by running the numerical models and the upstream water levels. The results showed that the error percentages between the total head estimated at the turbine model inlets and upstream water level of Haditha and Temenggor dams equal 2.16% and 0.3%, respectively. The reason for this minor percentage of errors due to the head loss varies according to multiple forms of turbulent flow and the types of pipes (smooth or rough). In addition, the rotational speed of the turbine varies according to two variables (the head and discharge) obtained from the data source, where no clear relationship was found. The comparison of the
total head at the turbine model inlets and the upstream water level indicates a practical procedure to find the accuracy of the model results.

Figure 4(a) shows the velocity distribution in Haditha turbine model with the maximum upstream water level = 150.2 m from the entrance up to the spiral casing; it varies due to changes in the cross-sectional area based on the continuity equation. Maximum velocities occur around the turbine runner because of the contraction of the cross-sectional area, and flow is limited to the lower part of the draft tube and the outlet based on the amount of water flow. Figure 4(b) illustrates a constant velocity distribution at the penstock of the Temenggor turbine model with the maximum upstream water level of 248.42 m, because of the constant cross-sectional area. Velocity gradually increases from the spiral casing to the turbine runner because of the contraction of the cross-sectional area. Velocity is consistently distributed across the draft tube because of the rotational motion and the turbulent flow incident that occur following the running of the turbine. The results showed that the maximum water velocity occurred at the location of the turbine runner, namely 27.3 m/s, for a discharge of 165.5 m$^3$/s in the Haditha turbine and 40 m$^3$/s for a discharge of 100 m$^3$/s in the Temenggor turbine. Although the discharge in the Haditha turbine is more than that in the Temenggor turbine, the cross-sectional area of the Haditha turbine is larger than that of the Temenggor turbine.

On the other hand, Figure 4(c) and (d) indicate the boundary pressure distributions in the Haditha and Temenggor turbine models, respectively. Here, the pressure distributions are proportional to the inverse of the velocity distribution based on the energy equation. The minimum pressure values are achieved following the running of the turbine; however, they do not reach cavitation pressure [46]. The velocity and pressure results obtained in the current study are in harmony with the modeling results accomplished by [39,44,47].

Figure 5(a) and (b) display the wall shear stress distributions of the Haditha and Temenggor turbine units running under the maximum head, respectively. The maximum wall shear stress values are 0.56 kPa and 1.5 kPa, which account for 0.1% and 0.25% of the maximum wall pressure values, respectively. Consequently, wall shear stress values are ignored in the process of transporting boundary pressure from the turbine models to the dam models for determining the effect of the running turbine on the dynamic behavior of the embankment dams, because their values are insignificant compared with the pressure values. Moreover, they depend on the pipe type (smooth or rough) and flow, which cannot be clearly identified.

Table 5 presents a comprehensively detailed visualization of the attained results on the pressure

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**Figure 4.** (a) Velocity vector of fluid in the flow passage at $t = 4.80$ s in Haditha turbine unit. (b) Velocity flow lines at $t = 4.80$ s in Temenggor turbine unit. (c) Pressure distribution of fluid in flow passage at $t = 4.80$ s in Haditha turbine unit. (d) Pressure distribution of fluid in flow passage at $t = 4.80$ s in Temenggor turbine unit.
were displayed graphically for Haditha and Temenggor turbine models, respectively. In Figure 6, the flow velocity phenomenon of Haditha turbine was simulated in accordance with two different dam components at the upstream water level and submerged weir depth. Based on Figure 6, it can be recognized that the velocity distribution on both sides of the draft tube became more regular with an increase in the submerged weir height. The maximum velocity range located in the turbine runner region and affected by the running turbine at the minimum and maximum upstream water levels with different submerged weir heights varies 7.6% and 4.1%, respectively. In terms of the water pressure value, Figure 7 indicates the water pressure fluctuation as well as pressure fluctuation in the dam’s upstream water level and submerged weir depth. Based on the obtained results, building 1.333 m submerged weirs in the downstream of Haditha turbine units with the running turbine model at the minimum (129 m) and maximum upstream water levels (150.2 m) reduced the pressure difference range between left and right sides by 23% and 1% from the total head, respectively.

Further, Temenggor turbine was modeled to investigate the same two interesting measures of water velocity and pressure. Figure 8 presents the effect of the upstream water level and the proposed weir height to optimize suitable steady water flow. Based on the graphical visualization, the optimal velocity distribution was attained when a 1-m submerged weir was built in the downstream of the turbine outlet. The maximum velocity range located in the turbine running region and affected by the running turbine at the minimum (236.5 m) and maximum (248.42 m) upstream water levels with changing built submerged weir height varies 26.7% and 10.9%, respectively. Water flow pressure is properly demonstrated in Figure 9. According to this figure, it can be concluded that running the turbine model at the minimum (236.5 m) and maximum upstream water levels (248.42 m) reduces the range of pressure differences between left and right sides by 8.5% and 15.9% from the total head, respectively.

From the engineering perspective, the cost of initiating a submerged weir is a very important element that needs to be considered by decision-makers. Note that constructing the submerged weir is not an easy task. It requires careful investigation, inspection, and optimized structure building. This might be costly in economic terms; yet, it is a great proposition for the dam sustainability and stability and hydropower operation.

In conclusion, results showed that a reduction in pressure fluctuation provides a uniform velocity distribution according to the Reynolds-averaged Navier-Stokes equations at the draft tubes, especially for the high discharge rates. This fluctuation is more visible in the Temenggor Francis turbine than in the Haditha.
Table 5. Draft-tube pressure results of different suggested weir heights in Haditha Kaplan turbine units and Temenggor Francis turbine unit.

<table>
<thead>
<tr>
<th>Haditha Kaplan turbine</th>
<th>The top surface of cone and draft tube connection</th>
<th>Left side</th>
<th>Right side</th>
<th>Difference in pressure head between L &amp; R sides (m)</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>U/S.W. L (m)</td>
<td>Head (m)</td>
<td>p (Pa)</td>
<td>p/γ (m)</td>
</tr>
<tr>
<td>Without weir</td>
<td>150.2</td>
<td>46.5</td>
<td>-37526</td>
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<td></td>
<td>139.6</td>
<td>32.5</td>
<td>-14033</td>
<td>-1.4305</td>
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<td></td>
<td>129</td>
<td>18.5</td>
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<td>-0.5789</td>
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<tr>
<td>Weir height = 1.333 m*</td>
<td>150.2</td>
<td>46.5</td>
<td>-12411</td>
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<tr>
<td></td>
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<td></td>
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<td>Weir height = 2.667 m</td>
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<td>3.36524</td>
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<tr>
<td></td>
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<td></td>
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<tr>
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</table>

<table>
<thead>
<tr>
<th>Temenggor Francis turbine</th>
<th>The top surface of cone and draft tube connection</th>
<th>Left side</th>
<th>Right side</th>
<th>Difference in pressure head between L &amp; R sides (m)</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>U/S.W. L (m)</td>
<td>Head (m)</td>
<td>p (Pa)</td>
<td>p/γ (m)</td>
</tr>
<tr>
<td></td>
<td>242.46</td>
<td>100.46</td>
<td>-79352</td>
<td>-8.0889</td>
</tr>
<tr>
<td></td>
<td>236.5</td>
<td>94.5</td>
<td>-34499</td>
<td>-3.5167</td>
</tr>
<tr>
<td>Weir height = 0.5 m</td>
<td>248.42</td>
<td>106.42</td>
<td>-13195</td>
<td>-13.451</td>
</tr>
<tr>
<td></td>
<td>242.46</td>
<td>100.46</td>
<td>-49123</td>
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<td>236.5</td>
<td>94.5</td>
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<tr>
<td>Weir height = 1 m*</td>
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<tr>
<td></td>
<td>242.46</td>
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<td></td>
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<tr>
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<td></td>
<td>236.5</td>
<td>94.5</td>
<td>2867</td>
<td>0.29225</td>
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</table>

*Indicate the optimal weir height.
Figure 6. Flow velocity magnitude in Haditha turbine unit.
Figure 7. Flow pressure magnitude in Haditha turbine unit.
Figure 8. Flow velocity magnitude in Temenggor turbine unit.
Figure 9. Flow pressure magnitude in Temenggor turbine unit.
Kaplan turbine due to the operation under high water level of the upstream (Table 1). The recommended submerged weir construction at the outlet draft tube results from the uneven distribution of water flow, particularly in the diffuser section. The findings indicate an essential solution that can be implemented practically in the powerhouse system operation in order to maintain a steady draft-tube water flow with balanced water pressures on both sides of the outlet. The proposed submerged weir can be further extended to envisage the instabilities of turbine outlet components of the prototype off-design operation.

7. Conclusions

A draft tube is an essential component of the powerhouse system that is located in the downstream part of any type of reaction turbines. The main goal of this draft tube is to convert the high-impact kinetic energy at the turbine runner outlet into pressure energy gradually. However, the off-design of draft tube initiates turbulent and fluctuating pressures during online operation; hence, solving this problem is extremely significant for the sustainability of dam body. This research provides a detailed analysis of the characteristics of the vertical Kaplan and Francis turbines influenced by changing water levels and discharge ranges. The actual pressure of the water flowing through the draft tube was computed qualitatively and presented numerically to verify the operation and performance of the turbines. The main determined pressures were obtained based on the proposition of a regulated submerged weir with different depths. The pressure distribution of unsteady flow was predicted through time-dependent running turbines. The change in the upstream water level head provides a guideline for flow characteristics in the turbine units. An increase in head per unit discharge is the main guideline for generating hydroelectric power. The construction of the power station far from the dam body increases the length of the waterway, leading to increased energy loss. However, a dam is protected from powerhouse vibration. The results of the proposed constructed submerged weir showed a very reasonable and reliable draft-tube turbine operation with very regular pressures. The Kaplan draft-tube turbine fits 1.33 m submerged weir height that comprises 16.7% of the total opening. On the other hand, the Francis draft-tube turbine was designed optimally with a 1 m submerged weir height that represents 33.3% of the total tube open depth. The intended suggestion was very sufficient and feasible for minimizing pressure fluctuation on both sides of the draft tubes of the investigated case studies. This research can be further extended to inspect the role of stressed components in the draft tube walls and propose a systematic hydropower operation based on the computed stresses.

References


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