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Nonlinear free vibration analysis of functionally graded plate resting on elastic foundation in thermal environment using higher-order shear deformation theory

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Abstract. This paper deals with nonlinear vibration analysis of a functionally graded plate resting on Pasternak elastic foundation in a thermal environment. A mathematical model is developed based on a higher-order shear deformation theory using Green-Lagrange type nonlinearity. The model includes all the nonlinear terms to obtain a general form and to present the original flexure of the plate. The material properties are considered as temperature dependent and graded along thickness direction following a simple power law distribution in terms of volume fraction of the constituents. The compression/traction free condition is employed to obtain a simplified model with seven parameters instead of nine parameters. The plate model has been discretized into C^0 eight-noded quadratic elements with seven degrees of freedom per node. The governing equation of the functionally graded plate has been derived using Hamilton's principle and solved by a direct iterative method. The present model is validated by comparing the obtained results with those published in the literature. The effects of volume fraction index, aspect ratio, thickness ratio, support conditions, elastic foundation modulus, and temperature on the nonlinear frequencies of the functionally graded plates are discussed. It has been found that the intermediate material property does not necessarily give intermediate nonlinear frequency.

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1. Introduction

The Functionally Graded Material (FGM), well known as the high-performance heat-resistant material, was proposed in 1984 by a group of material scientists of Sendai area in Japan. FGM is microscopically inhomogeneous composite material composed of ce-

ramic (the high-temperature side with good thermal resistance) and metal (the low-temperature side with superior fracture toughness), in which the material properties vary along the thickness direction obeying simple power law distribution that depends on the volume fraction of the constituent materials. Flat FGM plate subjected to an external load supported on the elastic foundation is a commonly used structure found in petrochemical, marine, aerospace, biomechanics as well as various mechanical, electrical, nuclear, and civil engineering. These structures are likely to undergo large amplitude vibration; thereby, their geometric

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nonlinear vibration analysis has gained considerable attention of the researchers since the last few decades.

Akhavan et al. [1,2] obtained the exact solution for the buckling load and frequencies of Mindlin plate supported on an elastic foundation subjected to uniform and linearly distributed in-plane loading. The analysis procedure includes the first-order shear deformation theory for the plate-foundation interaction model. Baferani et al. [3] employed an analytical approach to vibration analysis of an FGM rectangular plate resting on Pasternak elastic foundation using Hamilton's principle for the Levy type boundary conditions. They considered the displacement field based on the third-order shear deformation theory. Bodaghi and Saidi [4] achieved an exact analytical solution for the buckling of the FGM plate using classical plate theory and derived the governing equations based on the exact neutral surface position. Chien and Chen [5] studied the nonlinear vibration of the laminated plate resting on a nonlinear Winkler-type elastic foundation. Galerkin method and the Runge-Kutta method were employed to calculate the frequency ratio of an initially stressed laminated plate. Governing equations were derived using the variational principle. Duc et al. [6] calculated the numerical results of nonlinear thermal dynamic response and nonlinear vibration of thick FGM plates using a third-order shear deformation theory and stress function by the Runge-kutta method. Gajendar [7] employed a modified Galerkin method to derive a nonlinear differential equation in terms of elliptic function. Kiani et al. [8] presented a thermoelastic free vibration analysis of an FGM doubly curved panel using analytical hybrid Laplace-Fourier transformation based on the first-order shear deformation theory. Further, the analysis of the FGM doubly curved panel resting on the Pasternak-type elastic foundation was carried out by Kiani et al. [9]. Shen et al. [10] and Shen and Wang [11] presented a nonlinear vibration analysis of an FGM doubly curved panel resting on the elastic foundation in thermal environment based on the higher-order shear deformation theory and von Karman strain-displacement relationship, considering the micromechanics model like Voigt and Mori-Tanaka micromechanics model. Shen and Wang [12] presented the nonlinear vibration of the FGM cylindrical plate containing piezoelectric layers resting on the elastic foundation in a thermal environment based on the higher-order shear deformation theory. Huang et al. [13] presented an exact three-dimensional elasticity solution for the bending behavior of the FGM thick plate resting on Winkler-Pasternak elastic foundation using a state-space method in order to express the equations to condense first-order differential equations. Dehghan and Baradaran [14] proposed a coupled FE-DQ for 3D analysis of the thick rectangular plate resting on the elastic foundation and discretized the

plate geometry into 2D finite-element mesh. Qin and Diao [15] developed Hybrid-Trefftz (HT) p-element for nonlinear analysis of Reissner-Mindlin plate resting on elastic foundation. Duc and Cong [16] investigated the nonlinear post-buckling behavior of symmetric S-FGM plate resting on elastic foundation and subjected to thermo-mechanical loads using the Galerkin method. The model was developed in the framework of Reddy's third-order shear deformation plate theory and von-Karman type nonlinear equations. Fallah et al. [17] studied the free vibration analysis of moderately thick functionally graded plates supported on Winkler foundation using an extended Kantorovich method along with an infinite power series solution. Further, they presented nonlinear free vibration analysis of functionally graded beam resting on nonlinear elastic foundation using Galerkin one-parameter solution with von-Karman's strain displacement relation [18]. Taczala et al. [19] investigated the geometric nonlinear free vibration of the thick FGM plate in the thermal environment that is supported on a two-parameter foundation using FSDT and von Karman nonlinearity. Yang et al. [20] obtained the theoretical solution for a rectangular plate resting on elastic foundation with free edges by a reciprocal theorem method. Sundararajan et al. [21] employed von-Karman nonlinearity-based FSDT formulation to carry out the vibration analysis of the FGM plate in the thermal environment. Huang and Shen [22] presented the nonlinear vibration analysis of the FGM plate by incorporating HSDT that includes von-Karman-type nonlinearity. The nonlinear equations were solved by the improved perturbation technique. Civalek [23] employed the Discrete Singular Convolution (DSC), such as the Regularized Shanon's Kernel (RSK) and Lagrange Delta (LD) kernel, and a Harmonic Differential Quadrature (HDQ) method in order to derive the governing nonlinear partial differential equation of the plate on Winkler-Pasternak elastic foundation. Qin [24] presented the nonlinear analysis of Reissner plate resting on an elastic foundation by the boundary element method. Singha and Daripa [25] investigated the large amplitude flexural vibration of a composite plate subjected to periodic in-plane load employing HSDT with von-Karman nonlinearity. Thi and Duc [26] investigated the nonlinear stability of an FGM spherical shell on the elastic foundation in the thermal environment using FSDT and Galerkin method. Tornabene et al. [27] investigated the statics and dynamics of laminated doubly curved shells resting on Winkler-Pasternak elastic foundation using FSDT in conjunction with generalized differential quadrature. Tornabene et al. [28] proposed a 2D higher-order equivalent single-layer theory to determine the free vibration of doubly curved laminated composite shells with different curvatures. Szekrenyes [29,30] introduced the theorem of autocontinuity to attain continuity

between the laminated and delaminated portions and proposed the method of four equivalent single layers for modeling a delaminated orthotropic composite plate using second- and third-order laminated plate theories.

According to the above review, it is clear that many studies have already been reported on the nonlinear vibration behavior of FGM plates on elastic foundation using von-Karman-type nonlinear strain in the framework of FSDT and HSDT mid-plane kinematics that solely considers the transverse displacement. Based on this author's knowledge, no study has been carried out in open literature on geometric nonlinear free vibration behavior of the FGM plate resting on the two-parameter elastic foundation based on HSDT mid-plane kinematics and Green-Lagrange nonlinearity, considering moderately large displacements and rotations, including temperature-dependent material properties, due to the nonlinear temperature rise. Hence, the present study aims to predict an accurate geometric nonlinear analysis by developing a mathematical model that includes the Green-Lagrange nonlinear strains in the framework of HSDT. Based on HSDT, a modified displacement field with C^0 continuity has been employed that satisfies the traction-free boundary condition. The Green-Lagrange nonlinearity includes all nonlinear higher-order terms that have been incorporated to attain the original flexure and enhance the flexibility of the plate (general case) structure resting on the two-parameter elastic foundation in the thermal environment. The governing equations are obtained through Hamilton's principle, and the preferred nonlinear responses are computed by the direct iterative method. The plate has been discretized using an eight-noded quadratic serendipity element with seven degrees of freedom per node. The model has been validated by comparing it with results available in the literature. The effects of various parameters such as volume fraction index, aspect ratio, thickness ratio, foundation modulus (K_w , K_s), temperature, and different boundary conditions on frequency parameters are discussed in detail.

2. Material property of FGM

Consider an FGM plate with geometric dimensions of length a , width b , and thickness h resting on a two-parameter elastic foundation (a combination of an uncoupled spring layer and a shear layer at its top), as shown in Figure 1.

According to the power law distribution, the volume fraction of the ceramic constituent (top) can be written as follows:

$$V_c = \left(\frac{z}{h} + 0.5 \right)^n, \quad (1)$$

where z is the thickness co-ordinate ($-h/2 \leq z \leq h/2$), and n is the power law index ($0 \leq n \leq \infty$)

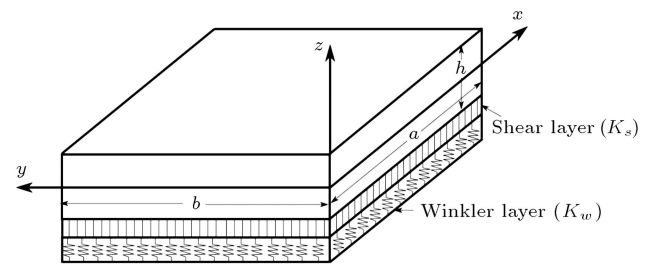


Figure 1. A functionally graded material plate resting on Pasternak foundation.

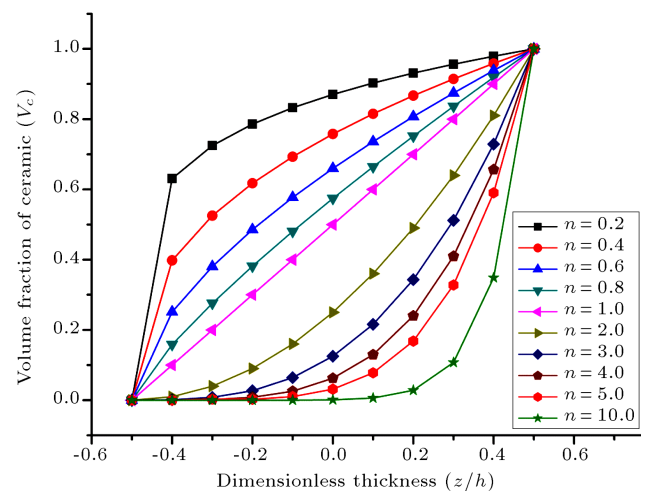


Figure 2. Variation of ceramic volume fraction, V_c , through the dimensionless thickness (Z/h).

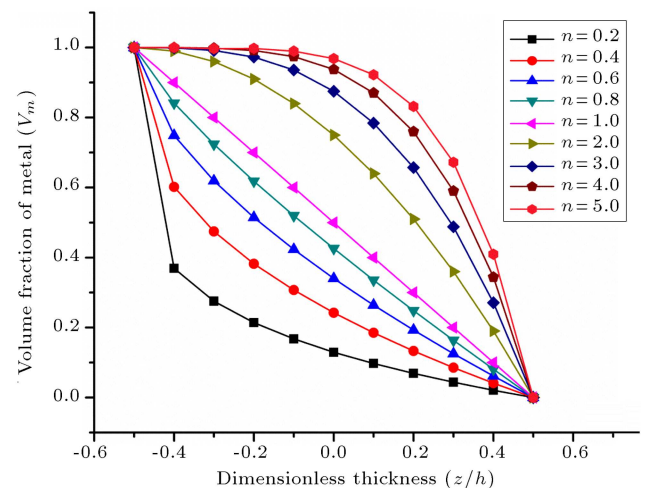


Figure 3. Variation of metal volume fraction, V_m , through the dimensionless thickness (Z/h).

responsible for generating an infinite number of varying compositions between two phases. The effect of power law index on the volume fraction of ceramic and metal is shown in Figures 2 and 3.

The effective properties such as Young's modulus, E_{ef} , Poisson's ratio, ν_{ef} , thermal expansion coefficient, α_{ef} , mass density, ρ_{ef} , and thermal conductivity, k_{ef} ,

are temperature dependent and are expressed in a non-linear function of temperature [31] as per the rule of mixture (Voigt rule).

$$P_{ef} = P_b(T) + [P_t(T) - P_b(T)] \left(\frac{z}{h} + \frac{1}{2} \right)^n, \quad (2)$$

where P_t and P_b are the properties of constituents at the top and the bottom of the plate. In this paper, metal is at the bottom ($z = -h/2$), and ceramic is at the top ($z = +h/2$).

$$P_m(T) \quad \text{and} \quad P_c(T) \\ = P_0(P_{-1}T^{-1} + 1 + P_1T + P_2T^2 + P_3T^3), \quad (3)$$

where P_0 , P_{-1} , P_1 , P_2 , and P_3 are the coefficients of temperature T (in K) and are unique for the constituent materials. The temperature-dependent material properties (E , α , ρ , k) can be obtained through Eqs. (4)-(7):

$$E(z, T) = E_m(T) + [E_c(T) - E_m(T)] \left(\frac{z}{h} + \frac{1}{2} \right)^n, \quad (4)$$

$$\alpha(z, T) = \alpha_m(T) + [\alpha_c(T) - \alpha_m(T)] \left(\frac{z}{h} + \frac{1}{2} \right)^n, \quad (5)$$

$$\rho(z, T) = \rho_m(T) + [\rho_c(T) - \rho_m(T)] \left(\frac{z}{h} + \frac{1}{2} \right)^n, \quad (6)$$

$$k(z, T) = k_m(T) + [k_c(T) - k_m(T)] \left(\frac{z}{h} + \frac{1}{2} \right)^n, \quad (7)$$

where subscripts m and c refer to metal and ceramic constituents, respectively.

3. Theoretical formulation

3.1. Displacement field

The displacement field considering HSDT is assumed, as given in [29,30,32]:

$$\begin{aligned} u(x, y, z) &= u_0(x, y) + z\theta_y(x, y) + z^2\vartheta_x(x, y) \\ &\quad + z^3\psi_x(x, y), \\ v(x, y, z) &= v_0(x, y) - z\theta_x(x, y) + z^2\vartheta_y(x, y) \\ &\quad + z^3\psi_y(x, y), \\ w(x, y, z) &= w_0(x, y), \end{aligned} \quad (8)$$

where u , v , and w represent the displacements of the point along the (x, y, z) coordinates. u_0 , v_0 , w_0 and θ_x , θ_y denote the in-plane displacements and the rotations of transverse normal to the midplane about x and y axes, respectively. Functions ϑ_x , ϑ_y , ψ_x and ψ_y are the higher-order terms in Taylor series expansion defined in the mid-plane of the plate.

A modified displacement model can be derived by implementing the assumption of zero transverse shear stress (τ_{xz} , τ_{yz}) across the interface (the traction-free condition). Hence, the shear strain is considered zero and is presented as follows:

$$\begin{aligned} \gamma_{xz} &= \frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} = \theta_y + 2z\vartheta_x + 3z^2\psi_x + \frac{\partial w_0}{\partial x} = 0, \\ \gamma_{yz} &= \frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} = -\theta_x + 2z\vartheta_y + 3z^2\psi_y + \frac{\partial w_0}{\partial y} = 0. \end{aligned} \quad (9)$$

By simplifying Eq. (8), the displacement model can be modified. Further, to obtain C^0 continuity, two new variables β_x and β_y are introduced by using a relation given in Appendix (Eq. (A.1)).

The modified displacement model with C^0 continuity is presented as follows:

$$\begin{aligned} u &= u_0 + z\theta_y - c_1z^3(\theta_y + \beta_x), \\ v &= v_0 - z\theta_x - c_1z^3(-\theta_x + \beta_y), \\ w &= w_0, \end{aligned} \quad (10)$$

where $c_1 = 4/3h^2$ and u , v , w , θ_x , θ_y , β_x , and β_y are considered as field variables for structural deformation.

3.2. Nonlinear temperature rise

The temperature distribution along the thickness direction can be obtained by solving a one-dimensional steady-state heat transfer equation. The steady-state heat conduction equation for the temperature through the thickness is given by:

$$-\frac{d}{dz} \left(k(z, T) \frac{dT}{dz} \right) = 0. \quad (11)$$

Further, Eq. (11) can be solved by imposing the boundary condition of $T = T_m$ at $z = -h/2$ and $T = T_c$ at $z = h/2$.

The variation of temperature in an FGM plate through the thickness is expressed as follows:

$$T(z) = T_m - (T_c - T_m) \frac{\int_{-h/2}^z (1/k(z, T)) dz}{\int_{-h/2}^{h/2} (1/k(z, T)) dz}. \quad (12)$$

The solution to the above equation as obtained by using polynomial series is as follows:

$$\begin{aligned} T(z) &= T_m + \frac{\Delta T}{\bar{P}} \left[\left(\frac{z}{h} + \frac{1}{2} \right) \right. \\ &\quad \left. - \frac{k_{cm}}{(n+1)k_m} \left(\frac{z}{h} + \frac{1}{2} \right)^{n+1} \right. \\ &\quad \left. + \frac{k_{cm}^2}{(2n+1)k_m^2} \left(\frac{z}{h} + \frac{1}{2} \right)^{2n+1} \right] \end{aligned}$$

$$\begin{aligned}
& - \frac{k_{cm}^3}{(3n+1)k_m^3} \left(\frac{z}{h} + \frac{1}{2} \right)^{3n+1} \\
& + \frac{k_{cm}^4}{(4n+1)k_m^4} \left(\frac{z}{h} + \frac{1}{2} \right)^{4n+1} \\
& - \frac{k_{cm}^5}{(5n+1)k_m^5} \left(\frac{z}{h} + \frac{1}{2} \right)^{5n+1} \Big], \quad (13)
\end{aligned}$$

where:

$$\begin{aligned}
\bar{P} = 1 - \frac{k_{cm}}{(n+1)k_m} + \frac{k_{cm}^2}{(2n+1)k_m^2} - \frac{k_{cm}^3}{(3n+1)k_m^3} \\
+ \frac{k_{cm}^4}{(4n+1)k_m^4} - \frac{k_{cm}^5}{(5n+1)k_m^5}, \quad (14)
\end{aligned}$$

and:

$$k_{cm} = k_c - k_m.$$

3.3. Nonlinear formulation

The strain developed in the FGM plate is the sum of linear and nonlinear strains. The nonlinear strain presents the Green-Lagrange strain displacement relation associated with moderately large displacements, and rotations include all the field variables as shown in relations given below:

$$\{\varepsilon\} = \{\varepsilon_L\} + \{\varepsilon_{NL}\}. \quad (15)$$

The above equation can be presented in the form of linear and nonlinear parts:

$$\begin{aligned}
\begin{Bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{Bmatrix} &= \begin{Bmatrix} u_{,x} \\ v_{,y} \\ u_{,y} + v_{,x} \\ u_{,z} + w_{,x} \\ v_{,z} + w_{,y} \end{Bmatrix} \\
&+ \begin{Bmatrix} \frac{1}{2} \{ (u_{,x})^2 + (v_{,x})^2 + (w_{,x})^2 \} \\ \frac{1}{2} \{ (u_{,y})^2 + (v_{,y})^2 + (w_{,y})^2 \} \\ \{ (u_{,x})(u_{,y}) + (v_{,x})(v_{,y}) + (w_{,x})(w_{,y}) \} \\ \{ (u_{,x})(u_{,z}) + (v_{,x})(v_{,z}) + (w_{,x})(w_{,z}) \} \\ \{ (u_{,y})(u_{,z}) + (v_{,y})(v_{,z}) + (w_{,y})(w_{,z}) \} \end{Bmatrix}. \quad (16)
\end{aligned}$$

By substituting Eq. (10) in Eq. (16), the strain displacement relation of the plate can be expressed as follows:

$$\{\varepsilon_L\} = \begin{Bmatrix} \varepsilon_1^0 \\ \varepsilon_2^0 \\ \varepsilon_6^0 \\ \varepsilon_5^0 \\ \varepsilon_4^0 \end{Bmatrix} + z \begin{Bmatrix} \kappa_1^1 \\ \kappa_2^1 \\ \kappa_6^1 \\ \kappa_5^1 \\ \kappa_4^1 \end{Bmatrix} + z^2 \begin{Bmatrix} \kappa_1^2 \\ \kappa_2^2 \\ \kappa_6^2 \\ \kappa_5^2 \\ \kappa_4^2 \end{Bmatrix} + z^3 \begin{Bmatrix} \kappa_1^3 \\ \kappa_2^3 \\ \kappa_6^3 \\ \kappa_5^3 \\ \kappa_4^3 \end{Bmatrix},$$

$$\begin{aligned}
\{\varepsilon_{NL}\} &= \frac{1}{2} \begin{Bmatrix} \varepsilon_1^{nl0} \\ \varepsilon_2^{nl0} \\ \varepsilon_6^{nl0} \\ \varepsilon_5^{nl0} \\ \varepsilon_4^{nl0} \end{Bmatrix} + \frac{z}{2} \begin{Bmatrix} \kappa_1^{nl1} \\ \kappa_2^{nl1} \\ \kappa_6^{nl1} \\ \kappa_5^{nl1} \\ \kappa_4^{nl1} \end{Bmatrix} \\
&+ \frac{z^2}{2} \begin{Bmatrix} \kappa_1^{nl2} \\ \kappa_2^{nl2} \\ \kappa_6^{nl2} \\ \kappa_5^{nl2} \\ \kappa_4^{nl2} \end{Bmatrix} + \frac{z^3}{2} \begin{Bmatrix} \kappa_1^{nl3} \\ \kappa_2^{nl3} \\ \kappa_6^{nl3} \\ \kappa_5^{nl3} \\ \kappa_4^{nl3} \end{Bmatrix} \\
&+ \frac{z^4}{2} \begin{Bmatrix} \kappa_1^{nl4} \\ \kappa_2^{nl4} \\ \kappa_6^{nl4} \\ \kappa_5^{nl4} \\ \kappa_4^{nl4} \end{Bmatrix} + \frac{z^5}{2} \begin{Bmatrix} \kappa_1^{nl5} \\ \kappa_2^{nl5} \\ \kappa_6^{nl5} \\ \kappa_5^{nl5} \\ \kappa_4^{nl5} \end{Bmatrix} \\
&+ \frac{z^6}{2} \begin{Bmatrix} \kappa_1^{nl6} \\ \kappa_2^{nl6} \\ \kappa_6^{nl6} \\ \kappa_5^{nl6} \\ \kappa_4^{nl6} \end{Bmatrix}. \quad (17)
\end{aligned}$$

The individual strain terms are described in Appendix (Eq. (A.2)).

$$\{\bar{\varepsilon}_L\} = \begin{Bmatrix} \varepsilon_1^0, \varepsilon_2^0, \varepsilon_6^0, \varepsilon_5^0, \varepsilon_4^0, \kappa_1^1, \kappa_2^1, \kappa_6^1, \kappa_5^1, \kappa_4^1, \\ \kappa_1^2, \kappa_2^2, \kappa_6^2, \kappa_5^2, \kappa_4^2, \kappa_1^3, \kappa_2^3, \kappa_6^3, \kappa_5^3, \kappa_4^3 \end{Bmatrix}, \quad (18)$$

and:

$$\{\bar{\varepsilon}_{NL}\} = \frac{1}{2} \begin{Bmatrix} \varepsilon_1^{nl0}, \varepsilon_2^{nl0}, \varepsilon_6^{nl0}, \varepsilon_5^{nl0}, \varepsilon_4^{nl0}, \kappa_1^{nl1}, \\ \kappa_2^{nl1}, \kappa_6^{nl1}, \kappa_5^{nl1}, \kappa_4^{nl1}, \kappa_1^{nl2}, \kappa_2^{nl2}, \\ \kappa_6^{nl2}, \kappa_5^{nl2}, \kappa_4^{nl2}, \kappa_1^{nl3}, \kappa_2^{nl3}, \kappa_6^{nl3}, \\ \kappa_5^{nl3}, \kappa_4^{nl3}, \kappa_1^{nl4}, \kappa_2^{nl4}, \kappa_6^{nl4}, \kappa_5^{nl4}, \\ \kappa_4^{nl4}, \kappa_1^{nl5}, \kappa_2^{nl5}, \kappa_6^{nl5}, \kappa_5^{nl5}, \kappa_4^{nl5}, \\ \kappa_1^{nl6}, \kappa_2^{nl6}, \kappa_6^{nl6}, \kappa_5^{nl6}, \kappa_4^{nl6} \end{Bmatrix}. \quad (19)$$

$\{\bar{\varepsilon}_L\}$ and $\{\bar{\varepsilon}_{NL}\}$ are the mid-plane linear and nonlinear strain terms. Superscripts 0, 1 and $nl0$, $nl1$ denote membrane and bending terms; 2-3 and $nl2$ – $nl6$ represent higher-order terms, respectively.

3.4. Thermo-elastic constitutive relations

The constitutive equation for the FGM plate subjected to thermal load is presented as follows:

$$\{\sigma\} = [\bar{Q}] \{\varepsilon - \alpha \Delta T\}, \quad (20)$$

where $\{\sigma\}$ and $\{\varepsilon\}$ are the stress and strain vectors, $[\bar{Q}]$ is the transformed reduced elastic stiffness matrix, ΔT is the increment of the temperature of the ceramic interface over reference temperature T_0 , and α is the coefficient of the thermal expansion.

Further, Eq. (20) can be expanded and rewritten as follows:

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{Bmatrix} = \begin{bmatrix} Q_{11} & Q_{12} & 0 & 0 & 0 \\ Q_{21} & Q_{22} & 0 & 0 & 0 \\ 0 & 0 & Q_{66} & 0 & 0 \\ 0 & 0 & 0 & Q_{44} & 0 \\ 0 & 0 & 0 & 0 & Q_{55} \end{bmatrix} \begin{Bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{Bmatrix} - \begin{Bmatrix} 1 \\ 1 \\ 0 \\ 0 \\ 0 \end{Bmatrix} \alpha(z, T) \Delta T, \quad (21)$$

where:

$$Q_{11} = Q_{22} = \frac{E(z, T)}{1 - \nu(z, T)^2},$$

$$Q_{12} = Q_{21} = \frac{\nu(z, T)E(z, T)}{1 - \nu(z, T)^2},$$

$$Q_{44} = Q_{55} = Q_{66} = \frac{E(z, T)}{2(1 + \nu(z, T))},$$

and ΔT is the temperature change for a stress-free state, $E(z, T)$ is the modulus, and $\alpha(z, T)$ is the coefficient of the thermal expansion.

4. FEM model

For the present analysis, the model is discretized using an eight-noded isoparametric quadratic element with seven degrees of freedom associated with each node for the finite element modeling. The field variables at any arbitrary point are related to the nodal field variables via shape functions N_i . The relation is presented below as in the study of Cook et al. [33]:

$$\{\delta^e\} = \{u_0^e, v_0^e, w_0^e, \beta_x^e, \beta_y^e, \theta_x^e, \theta_y^e\},$$

and:

$$\{\delta\} = \sum_{i=1}^8 N_i^e(\xi, \eta) \{\delta^e\}, \quad (22)$$

where $u_0^e, v_0^e, w_0^e, \beta_x^e, \beta_y^e, \theta_x^e, \theta_y^e$ are the field variables/degrees of freedom, $N_i^e(\xi, \eta)$ are the shape functions, and ξ, η are the natural coordinates.

5. The energy equations

5.1. Strain energy

The strain energy of the plate can be expressed as follows:

$$U = \frac{1}{2} \int_V \{\varepsilon\}^T \{\sigma\} dV. \quad (23)$$

The vectors of the strain component are:

$$\{\sigma\} = [\bar{Q}] \{\varepsilon\}, \quad (24)$$

$$\{\varepsilon\} = \left(B_L + \frac{1}{2} B_{NL} \right) \{\delta\}_e. \quad (25)$$

By substituting the strain and stress vectors from Eq. (24) and (25) in Eq. (23), the expression can be rewritten as follows:

$$U = \frac{1}{2} \int_A (\{\varepsilon\}^T [\bar{Q}] \{\varepsilon\}) dx dy = \frac{1}{2} \int_A (\varepsilon_L + \varepsilon_{NL})^T [\bar{Q}] (\varepsilon_L + \varepsilon_{NL}) dA. \quad (26)$$

The Eq. (26) can be further modified as follows:

$$U_s = \frac{1}{2} \int_A \left(\{\varepsilon_L\}^T [P_1] \{\varepsilon_L\} + \frac{1}{2} \{\varepsilon_L\}^T [P_2] \{\varepsilon_{NL}\} + \frac{1}{2} \{\varepsilon_{NL}\}^T [P_3] \{\varepsilon_L\} + \frac{1}{4} \{\varepsilon_{NL}\}^T [P_4] \{\varepsilon_{NL}\} \right) dA, \quad (27)$$

$$P_1 = \sum_{i=1}^8 \int_{-h/2}^{h/2} [H_L]^T [\bar{Q}] [H_L] dz,$$

$$P_2 = \sum_{i=1}^8 \int_{-h/2}^{h/2} [H_L]^T [\bar{Q}] [H_{NL}] dz,$$

$$P_3 = \sum_{i=1}^8 \int_{-h/2}^{h/2} [H_{NL}]^T [\bar{Q}] [H_L] dz,$$

$$P_4 = \sum_{i=1}^8 \int_{-h/2}^{h/2} [H_{NL}]^T [\bar{Q}] [H_{NL}] dz, \quad (28)$$

where $[H_L]$ and $[H_{NL}]$ are the thickness matrices given in Appendix (Eq. (A.2)), and $[P_1]$, $[P_2]$, $[P_3]$ and $[P_4]$ are the modified reduced elastic stiffness matrices as expressed in Eq. (28).

The total strain energy (U_{total}) can be expressed as follows:

$$U_{\text{total}} = (U_s + U_{ef}). \quad (29)$$

The elastic strain energy of the FGM plate (U_s) is:

$$\begin{aligned}
U_s = & \frac{1}{2} \left(\{\delta\}^{tr} \{B_L\}^{tr} [P_1] \{B_L\} \{\delta\} \right. \\
& + \frac{1}{2} \{\delta\}^{tr} \{B_L\}^{tr} [P_2] \{B_{NL}\} \{\delta\} \\
& + \frac{1}{2} \{\delta\}^{tr} \{B_{NL}\}^{tr} [P_3] \{B_L\} \{\delta\} \\
& \left. + \frac{1}{4} \{\delta\}^{tr} \{B_{NL}\}^{tr} [P_4] \{B_{NL}\} \{\delta\} \right). \quad (30)
\end{aligned}$$

The strain energy due to elastic foundation (U_{ef}) is:

$$U_{ef} = \frac{1}{2} \int_0^a \int_0^b \left[k_w w^2 + k_s \left(\left(\frac{\partial w}{\partial x} \right)^2 + \left(\frac{\partial w}{\partial y} \right)^2 \right) \right] dx dy, \quad (31)$$

where k_w and k_s are the Winkler and shear/Pasternak moduli.

5.2. Kinetic energy

The kinetic energy of an FGM plate is expressed as in the study of Cook et al. [33]:

$$\begin{aligned}
T = & \frac{1}{2} \int_{-h/2}^{h/2} \rho \left[(\dot{u})^2 + (\dot{v})^2 + (\dot{w})^2 \right] dA \\
= & \frac{1}{2} \left\{ \dot{\delta} \right\}^{tr} [M] \left\{ \dot{\delta} \right\}. \quad (32)
\end{aligned}$$

The mass matrix for the FGM plate resting on the elastic foundation is presented as follows:

$$[M] = \int_0^a \int_0^b [N]^{tr} [I] [N] dx dy = \int_{-1}^1 \int_{-1}^1 [N]^{tr} [I] [N] d\xi d\eta,$$

where $[I]$ and $[N]$ are the inertia and shape function matrix, respectively, as shown in Appendix (Eq. (A.3)).

5.3. Work done due to thermal load

Due to thermal load as well as non-uniform temperature rise along the temperature-dependent material properties, thermal stress tends to undergo geometric distortion. Due to a change in geometry, thermal stress is developed with a significant effect on the vibration characteristics. To calculate the solution based on the current transformed state of the structure, the second Piola-Kirchhoff stress has been considered in the direct iterative method of Green Lagrange-type nonlinearity. In this respect, it is required to calculate the geometric matrix due to the deformed geometry of the structure caused by thermal stress.

The stress developed due to the thermal effect can be presented as follows:

$$\left\{ \begin{matrix} N \\ M \\ P \end{matrix} \right\} = \left[\int_{-h/2}^{h/2} \{ [\bar{Q}] \{ \alpha(z, T) \} (1, z, z^3) \Delta T \} dz \right], \quad (33)$$

where:

$$\{ N \quad M \quad P \}^{tr} = [\{ N^T \}, \{ M^T \}, \{ P^T \}]^{tr},$$

are the in-plane thermal forces, moments, and higher-order terms, respectively.

The work done due to the resultant thermal force is the resultant work carried out by thermal force viz. W_T expressed as in the study of Cook et al. [33]:

$$W_T = \int_{-h/2}^{h/2} \{ \varepsilon_G \}^{tr} [S] \{ \varepsilon_G \} dA,$$

$$\{ \varepsilon_G \} = [H] [A_g] [\aleph] = [B_G] \{ \delta \}. \quad (34)$$

Further, Eq. (34) can be rewritten as follows:

$$W_T = \frac{1}{2} \int_A (\{ \delta \}^{tr} [B_G]^{tr} [S] [B_G] \{ \delta \}) dA, \quad (35)$$

where $\{ \varepsilon_G \}$ is the geometric strain vector, $[B_G]$ is the product form of a differential operator, and the corresponding matrices $[H]$, $[A_g]$, $[\aleph]$, $[S]$ are described, as shown in Eqs. (A.4), (A.5), (A.6), and (A.7) in the Appendix. The elemental geometric stiffness matrix $[K_G]$ for geometric distortion due to thermal load can be expressed as follows:

$$\begin{aligned}
[K_G] = & \int_0^a \int_0^b ([B_G]^{tr} [S] [B_G]) dx dy \\
= & \int_{-1}^1 \int_{-1}^1 ([B_G]^{tr} [S] [B_G]) d\xi d\eta. \quad (36)
\end{aligned}$$

6. The governing equation

The governing equations are obtained using Hamilton's principle:

$$\begin{aligned}
\int_0^t L dt = 0, \\
L = \delta \Pi = \delta [T - (U_s + U_f + W_{\Delta T})]. \quad (37)
\end{aligned}$$

By substituting Eqs. (30), (31), (32), and (35) in Eq. (37), the final form of the governing equation for nonlinear vibration can be presented as follows:

$$\begin{aligned}
[M] \{ \ddot{\delta} \} + \left([K_L] - [K_G] + \frac{1}{2} [K_{NL1}] + \frac{1}{3} [K_{NL2}] \right) \{ \delta \} \\
= 0. \quad (38)
\end{aligned}$$

The global stiffness matrix is composed of linear stiffness matrix $[K_L]$, global geometric stiffness matrix $[K_G]$, and nonlinear global coupled stiffness matrices $[K_{NL1}]$ and $[K_{NL2}]$ that linearly and quadratically depends on the displacement vector:

$$\begin{aligned}
[K_L] &= \int_{-1}^1 \int_{-1}^1 ([B_L]^{tr} [P_1] [B_L]) |J| d\xi d\eta, \\
[K_{NL1}] &= \frac{1}{2} \int_{-1}^1 \int_{-1}^1 ([B_L]^{tr} [P_2] [B_{NL}] \\
&\quad + [B_{NL}]^{tr} [P_3] [B_L]) |J| d\xi d\eta, \\
[K_{NL2}] &= \frac{1}{4} \int_{-1}^1 \int_{-1}^1 ([B_{NL}]^{tr} [P_4] [B_{NL}]) |J| d\xi d\eta, \\
[K_G] &= \int_{-1}^1 \int_{-1}^1 ([B_G]^{tr} [S] [B_G]) |J| d\xi d\eta. \quad (39)
\end{aligned}$$

In terms of eigenvalue and eigenvector form, Eq. (38) can be written as follows:

$$([K_L] - [K_G] + [K_{NL1}] + [K_{NL2}] - \omega^2 [M]) \Delta = 0, \quad (40)$$

where ω and Δ are the natural frequency and its corresponding eigenvector, respectively.

7. Methodology

Eq. (41) can be solved using the direct iteration method, and the step-wise procedure is stated below:

Step 1. Evaluate the elemental stiffness and mass matrices using general finite element procedures;

Step 2. Calculate the global stiffness and mass matrix by assembling the elemental matrices;

Step 3. Solve linear eigenvalue using the global stiffness and mass matrix:

$$([K] - \omega^2 [M]) \{\delta\} = \{0\},$$

Extract the linear eigenvalue and its corresponding eigenvector using a standard extraction algorithm;

Step 4. Scale up the amplitude ratio (W_{\max}/h) by the desired value and the ratio of maximum deflection of plate W_{\max} to the thickness of plate h and normalize the mode shape vector;

Step 5. Obtain the nonlinear stiffness matrices $[K_{NL1}]$ and $[K_{NL2}]$ using the scaled-up eigenvector (mode shape) by a numerical integration technique;

Step 6. Solve the nonlinear eigenvalue equation until the relative convergence criterion is 0.1%:

$$\frac{|\omega_i - \omega_{i-1}|}{\omega_i} \leq 0.1\%.$$

where i denotes the iteration number.

8. Validation

In order to validate the present method, the numerical results of $\text{Si}_3\text{N}_4/\text{SUS304}$ FGM plate are compared with the published results of the given literature. Table 1 shows the comparison of natural frequency parameters of a square $\text{Si}_3\text{N}_4/\text{SUS304}$ FGM plate of side 0.2 m and side-thickness ratio $a/h = 8$ subjected to three different thermal conditions along ceramic surface $T_c = 300$ K,

Table 1. Comparison of natural frequency parameters for $\text{Si}_3\text{N}_4/\text{SUS304}$ square plate in thermal environment: $a = b = 0.2$ m, and $h = 0.025$ m (TD: Temperature Dependent; TI: Temperature Independent).

Temperature	Source	Volume fraction index (n)				
		Ceramic	$n = 0.5$	$n = 1$	$n = 2$	Metal
$T_c = T_m = 300$ K	Present	12.5081	8.7169	7.6081	6.7374	5.4108
	Huang and Shen [22]	12.495	8.675	7.555	6.777	5.405
$T_c = 400$ K, $T_m = 300$ K (TD)	Present	12.3429	8.5815	7.4810	6.6154	5.2908
	Huang and Shen [22]	12.397	8.615	7.474	6.693	5.311
$T_c = 400$ K, $T_m = 300$ K (TI)	Present	12.3781	8.6010	7.4953	6.6258	5.2946
	Huang and Shen [22]	12.382	8.641	7.514	6.728	5.335
$T_c = 600$ K, $T_m = 300$ K (TD)	Present	12.0487	8.3147	7.2199	6.3558	5.0149
	Huang and Shen [22]	11.984	8.269	7.171	6.398	4.971
$T_c = 600$ K, $T_m = 300$ K (TI)	Present	12.1125	8.3628	7.2628	6.3947	5.0518
	Huang and Shen [22]	12.213	8.425	7.305	6.523	5.104

400 K, and 600 K. The reference temperature is $T_0 = T_m = 300$ K. The calculated frequency parameter (ω) is compared with the results reported by Huang and Shen [22]. The comparison shows good agreement. The volume fraction index $n = 0$ corresponds to ceramic and $n = 8$ corresponds to metal.

Table 2 shows the comparison of the non-dimensional natural frequency of simply supported square isotropic plate ($n = 0$) and values given by Baferani et al. [3]. Very good agreement can be observed for all boundary conditions. Table 3 presents the comparison of nonlinear frequency ratio of the

simply supported FGM plate and the values given in Sundararajan et al. [21]. The volume fraction of the FGM plate ($n = 1$) and side-thickness ratio $a/h = 10$. The comparison shows very good agreement. Hence, the present formulation can be trusted.

9. Results and discussion

A square $\text{Si}_3\text{N}_4/\text{SUS304}$ FGM plate composed of silicon nitride (ceramic) and steel (metal) supported on the elastic foundation has been considered for the parametric study. The side of the plate is 1 m and thickness is

Table 2. Comparison of non-dimensional natural frequency, ω , for homogeneous isotropic SSSS, SSSC, SCSC, and SSSF plates, $n = 0$.

Foundation parameters		Method	Boundary conditions			
K_w	K_s		SSSS	SSSC	SCSC	SSSF
0	0	Present	19.7350	23.6594	28.9950	11.6765
		Baferani et al. [3]	19.7374	23.6428	28.9441	11.6898
	100	Present	48.5473	51.2529	54.6165	37.1015
		Baferani et al. [3]	48.6149	51.3175	54.6742	37.1519
	1000	Present	140.1819	142.4385	144.8770	110.4241
		Baferani et al. [3]	141.8730	144.24	146.7191	111.7454
100	0	Present	22.1254	25.6867	30.6717	15.3732
		Baferani et al. [3]	22.1261	25.6706	30.6229	15.3834
	100	Present	49.5661	52.2199	55.5237	38.4263
		Baferani et al. [3]	49.6327	52.2827	55.5811	38.4742
	1000	Present	140.5379	142.7889	145.2219	110.8759
		Baferani et al. [3]	142.2250	144.5466	147.0595	112.1920
1000	0	Present	37.2744	39.4928	42.9024	33.7078
		Baferani et al. [3]	37.2763	39.4834	42.8686	33.7139
	100	Present	57.9363	60.2224	63.1093	48.7487
		Baferani et al. [3]	57.9945	60.2781	63.1603	48.7878
	1000	Present	143.7040	145.9058	148.2876	114.8621
		Baferani et al. [3]	145.3545	147.6268	150.0881	116.1336

Table 3. Comparison study of the nonlinear frequency ratio of a simply supported FGM plate at ambient temperature: $a/b = 1$, $a/h = 10$, and $n = 1$.

W_{\max}/h	$T_c = T_m = 300$ K		$T_c = 400$ K, $T_m = 300$ K		$T_c = 600$ K, $T_m = 300$ K	
	Sundararajan et al. [21]	Present	Sundararajan et al. [21]	Present	Sundararajan et al. [21]	Present
0.2	1.0063	1.0188	1.0076	1.0089	1.0107	1.0219
0.4	1.0654	1.0751	1.0702	1.0348	1.0818	1.0872
0.6	1.1707	1.1688	1.1802	1.1761	1.2037	1.1949
0.8	1.3155	1.3057	1.3267	1.3245	1.3638	1.3365
1.0	1.4789	1.4674	1.50	1.5493	1.5518	1.5578

Table 4. Temperature-dependent material properties of elastic modulus, E (GPa), Poisson's ratio, ν , coefficient of thermal expansion, $\alpha(1/K)$, density, ρ (kg/m³), and thermal conductivity, k , (W/mK) of ceramic and metal.

Material	P_0	P_{-1}	P_1	P_2	P_3	P (at 300 K)
Si₃N₄						
E	348.43	0	-3.070×10^{-13}	2.160×10^{-16}	-8.946×10^{-20}	322.2715
α	5.8723×10^{-6}	0	9.095×10^{-6}	0	0	7.4746×10^{-6}
ρ	2370	0	0	0	0	2370
k	9.19	0	0	0	0	9.19
SUS304						
E	201.04	0	3.079×10^{-13}	-6.534×10^{-16}	0	207.7877
α	12.330e-6	0	8.086e-6	0	0	15.321e-6
ρ	8166	0	0	0	0	8166
k	12.04	0	0	0	0	12.04

0.05 m. The Poisson's ratio, ν , is assumed to be 0.28. The two-parameter (Pasternak) elastic foundation has been considered. The material properties of ceramic and metal are shown in Table 4. The Poisson's ratio is taken as $\nu = 0.28$.

Non-dimensional frequency ($\bar{\omega}$) as given in Dehghan and Baradaran [14] is:

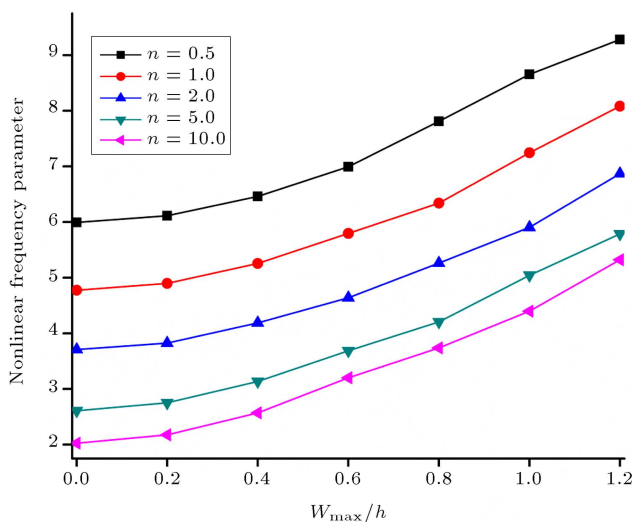
$$\bar{\omega} = \frac{\omega b^2}{\pi^2} \sqrt{\frac{\rho h}{D}}, \quad D = \frac{Eh^3}{12(1-\nu^2)}.$$

Non-dimensional elastic coefficients of Winkler and Pasternak are:

$$K_w = \frac{k_w a^4}{D}, \quad K_s = \frac{k_s a^2}{D}.$$

9.1. Effect of volume fraction index (n)

Figure 4 shows the effect of volume fraction index (n) on the frequency parameter of an FGM plate with

**Figure 4.** Variation of nonlinear frequency parameter for different volume fraction indexes (n).

a side-thickness ratio ($a/h = 40$), elastic foundation modulus coefficient $K_w = K_s = 100$, and the temperature along the ceramic side ($T_c = 400$ K) with a change in amplitude ratio. The linear and nonlinear frequencies decrease with an increase in the volume fraction index. With an increase in the value of volume fraction index, the ceramic content decreases, thereby decreasing the stiffness. Hence, the frequency parameter decreases.

9.2. Effect of aspect ratio (a/b) and side-thickness ratio (a/h)

Figure 5(a) shows the effect of aspect ratio on the nonlinear frequency parameter of the FGM plate with volume fraction index $n = 0.5, 1$, and 2 , side-thickness ratio $a/h = 20$, the elastic foundation modulus coefficient $K_w = K_s = 100$, and the temperature in ceramic side $T_c = 400$ K with a change in amplitude ratio. The amplitude ratio (W_{\max}/h) presents the degree of nonlinearity. W_{\max} presents the maximum transverse displacement with h , the thickness of the element. When $W_{\max}/h = 0$, it provides a complete linear condition and, with a simultaneous increase in amplitude, the nonlinearity increases. Hence, it is observed that, with an increase in aspect ratio, both the linear and nonlinear frequency parameters increase. Figure 5(b) shows the effect of side-thickness ratio (a/h) on the frequency parameter of the FGM plate ($a/h = 30$, $K_w = 10$, $K_s = 10$, and $\Delta T = 100$ K) with an increase in amplitude ratio. Both the linear and nonlinear frequency parameters decrease with an increase in the side-thickness ratio.

9.3. Effect of Winkler modulus (K_w) and Pasternak/shear modulus (K_s)

Figure 6 presents the influence of foundation modulus (K_w , K_s) on the nonlinear frequency parameter of Si₃N₄/SUS304 FGM plate ($n = 0.5, 1$ and 2 , $a/h = 40$,

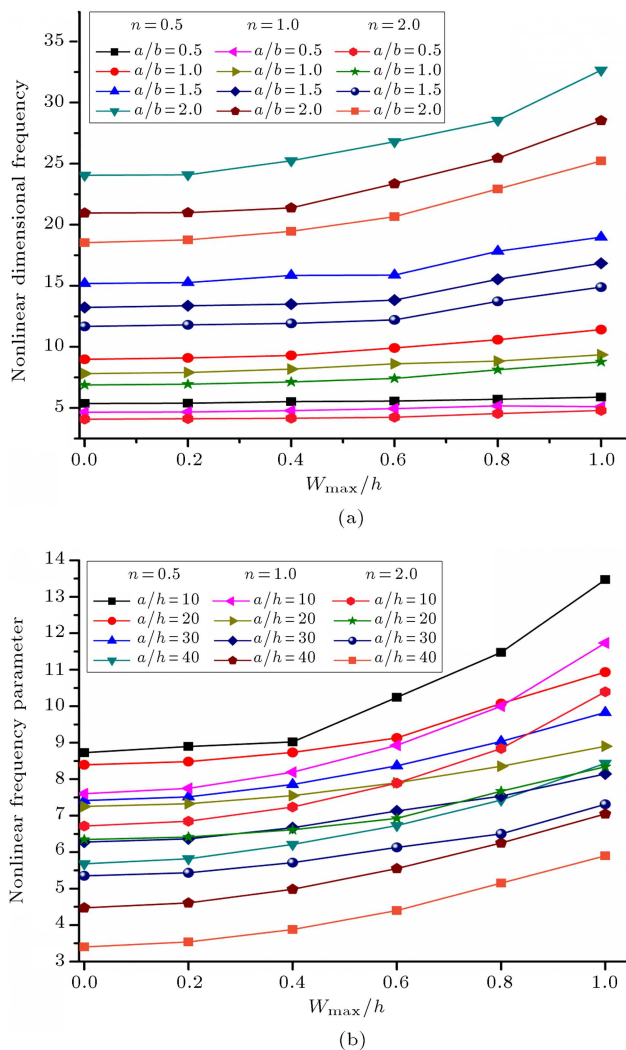


Figure 5. (a) Variation of nonlinear frequency parameter with varying aspect ratios. (b) Variation of nonlinear frequency parameter with varying side-thickness ratio.

$\Delta T = 100$ K) with a change in amplitude ratio. In Figure 6(a) the frequency parameter increases with an increase in the values of W_{\max}/h and K_w by keeping the shear modulus coefficient as constant, $K_s = 10$. In Figure 6(b) the frequency parameter increases with an increase in the values of K_s and W_{\max}/h by keeping the Winkler modulus coefficient as constant, $K_w = 10$. The frequency parameter for an FGM plate is higher if the plate rests on the elastic foundation. A small change in frequency parameter is noticed with the rise in Winkler modulus coefficient, whereas a significant change is noticed when the plate is supported on Pasternak/shear elastic foundation.

Figure 7 shows the effect of the elastic foundation modulus with Winkler modulus coefficient (K_w) and shear layer modulus coefficient (K_s) on the frequency parameter of the FGM plate with volume fraction index ($n = 1$), the side-thickness ratio ($a/h = 30, 40$), and temperature rise ($\Delta T = 100$ K). The reference

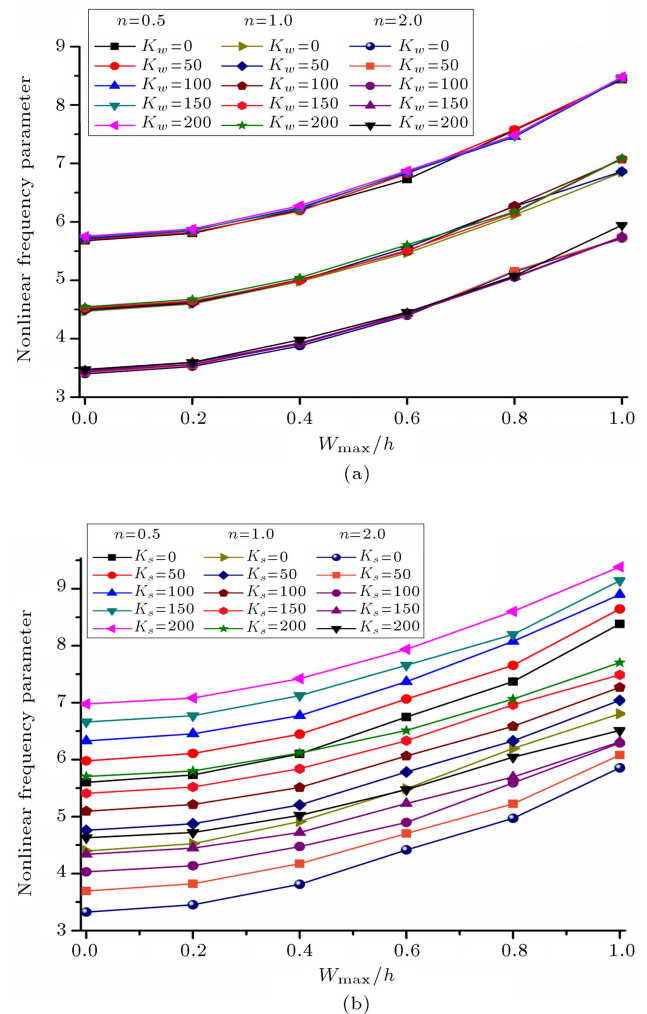


Figure 6. (a) Variation of the nonlinear frequency parameter for different Winkler modulus coefficients (K_w) with changes in the amplitude ratio. (b) Variation of nonlinear frequency parameter for different shear modulus coefficient (K_s) with change in amplitude ratio.

temperature is considered as 300 K along the metal side ($T_m = 300$ K). The plate supported on the elastic foundation has a higher frequency parameter than the plate without foundation. The shear layer modulus has a more significant effect on the fundamental frequency parameter than the Winkler modulus. The linear and nonlinear frequency parameters increase with an increase in Winkler modulus and, also, increases with an increase in shear layer modulus.

9.4. Effect of change in temperature

Figure 8 shows the variation of nonlinear frequency parameter with a change in nonlinear temperature along the ceramic side of the FGM plate with side-thickness ratio ($a/h = 30$) for different compositions ($n = 0.5, 1$ and 2). The two-parameter elastic foundation modulus consists of Winkler modulus coefficient $K_w = 50$ and shear modulus coefficient $K_s = 50$. The linear and nonlinear frequency parameters decrease with an increase

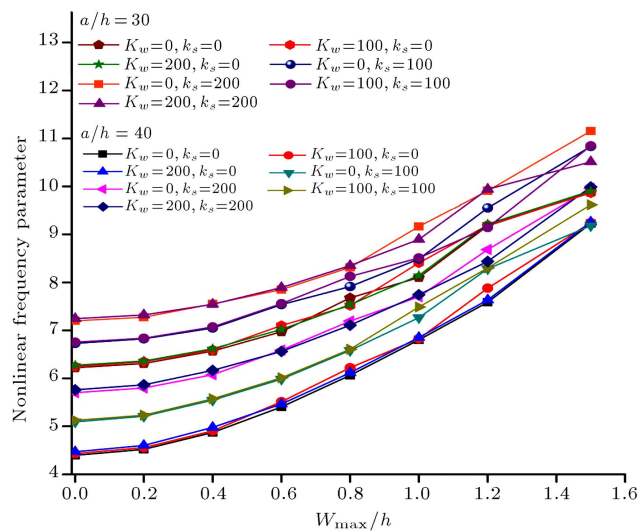


Figure 7. Nonlinear frequency parameter versus amplitude ratio with the varying elastic foundation modulus, K_w , K_s , at different side-thickness ratios.

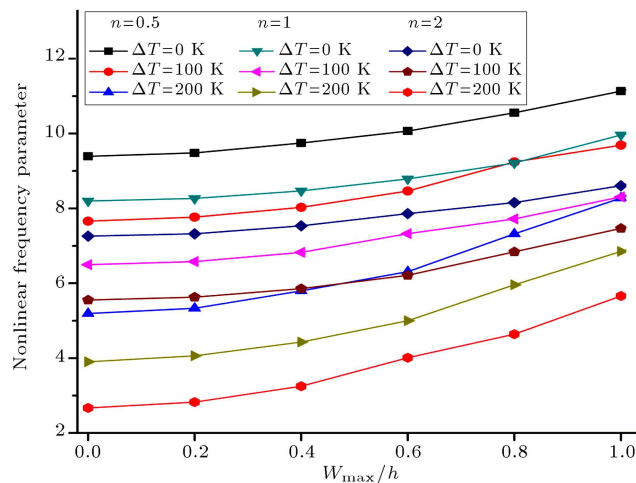


Figure 8. Nonlinear frequency parameter versus amplitude ratio with varying temperature rises.

in temperature rise (ΔT). At higher temperatures, the Young's modulus of the plate weakens and, thus, reduces the corresponding frequency and degrades the strength of the material, as shown in Figure 9.

9.5. Effect of boundary conditions

Figure 10 shows the effect of varying boundary conditions on frequency parameter for the FGM plate with different compositions $n = 0.5, 1, 2$, side-thickness ratio $a/h = 20$, elastic foundation modulus coefficient $K_w = K_s = 100$, and the temperature at ceramic side $T_c = 400$ K with a change in amplitude ratio. The FGM plate with all sides clamped has higher linear and nonlinear frequency parameters followed by SCSC (simply supported-clamped-simply supported-clamped) and SSSS (all sides are simply supported).

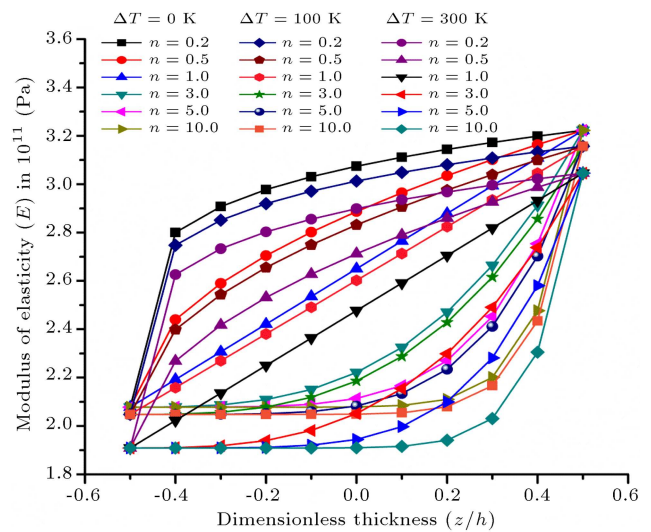


Figure 9. Variation of modulus of elasticity with changes in temperature increment and volume fraction index with varying dimensionless thicknesses.

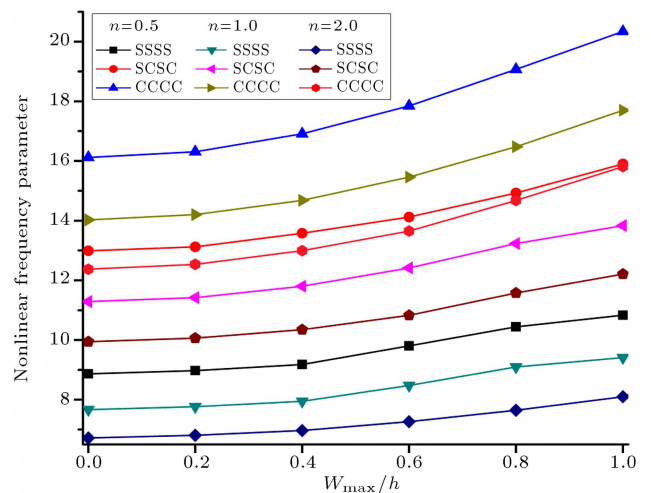


Figure 10. Variation of nonlinear frequency parameter with changes in boundary conditions.

The frequency increases with an increase in the number of clamped sides.

10. Conclusion

In this study, a numerical method was employed to obtain the nonlinear frequency parameters using modified HSDT with C^0 continuity. By considering Green-Lagrange nonlinearity, a general case was developed that includes all the nonlinear terms and predicts the original flexure of the plate. The nonlinear analysis showed hardening type nonlinear behavior that depends on the level of amplitude. The number of iterations involved to obtain the convergence of solution increases with increases in the amplitude ratio.

It was found that the intermediate material property did not necessarily give intermediate nonlinear

frequency. The conclusions drawn from the outcomes are:

- a. The shear layer modulus coefficient has a greater effect on the fundamental frequency parameter than Winkler modulus coefficient does. The linear and nonlinear frequency parameters of the FGM plate increased with an increase in Winkler coefficient and Pasternak/shear coefficient;
- b. The linear and nonlinear frequency parameters of the FGM plate resting on the elastic foundation increase with an increase in the aspect ratio and decrease with an increase in side-thickness ratio;
- c. The nonlinear fundamental frequency of the FGM plate resting on elastic foundation decreases with an increase in volume fraction index and in temperature along the ceramic surface;
- d. The nonlinear fundamental frequency increases with an increase in the number of clamped sides and decreases with an increase in the number of simply supported sides.

References

1. Akhavan, H., Hashemi, S.H., Taher, H.R.D., Alibeigloo, A., and Vahabi, S. "Exact solutions for rectangular Mindlin plates under in-plane loads resting on Pasternak elastic foundation. Part I: Buckling analysis", *Comput. Mater. Sci.*, **44**, pp. 968-978 (2009).
2. Akhavan, H., Hashemi, S.H., Taher, H.R.D., Alibeigloo, A., and Vahabi, S. "Exact solutions for rectangular Mindlin plates under in-plane loads resting on Pasternak elastic foundation. Part II: Frequency analysis", *Comput. Mater. Sci.*, **44**, pp. 951-961 (2009).
3. Baferani, A.H., Saidi, A.R., and Ehteshami, H. "Accurate solution for free vibration analysis of functionally graded thick rectangular plates resting on elastic foundation", *Compos. Struct.*, **93**, pp. 1842-1853 (2011).
4. Bodaghi, M. and Saidi, A.R. "Stability analysis of functionally graded rectangular plates under nonlinearly varying in-plane loading resting on elastic foundation", *Arch. Appl. Mech.*, **81**, pp. 765-780 (2011).
5. Chien, R.-D. and Chen, C.-S. "Nonlinear vibration of laminated plates on a nonlinear elastic foundation", *Compos. Struct.*, **70**, pp. 90-99 (2005).
6. Duc, N.D., Bich, D.H., and Cong, P.H. "Nonlinear thermal dynamic response of shear deformable FGM plates on elastic foundations", *J. Therm. Stress.*, **39**, pp. 278-297 (2016).
7. Gajendar, N. "Large amplitude vibration of plates on elastic foundations", *Int. J. Non-Linear Mech.*, **2**, pp. 163-172 (1967).
8. Kiani, Y., Shakeri, M., and Eslami, M.R. "Thermoelectric free vibration and dynamic behaviour of an FGM doubly curved panel via the analytical hybrid Laplace-Fourier transformation", *Acta Mech.*, **223**, pp. 1199-1218 (2012).
9. Kiani, Y., Akbarzadeh, A.H., Chen, Z.T., and Eslami, M.R. "Static and dynamic analysis of an FGM doubly curved panel resting on the Pasternak-type elastic foundation", *Compos. Struct.*, **94**, pp. 2474-2484 (2012).
10. Shen, H., Chen, X., Licheng, G., Wu, L., and Huang, X.-L. "Nonlinear vibration of FGM doubly curved panel resting on elastic foundations in thermal environments", *Aerosp. Sci. Technol.*, **47**, pp. 434-446 (2015).
11. Shen, H.S. and Wang, Z.X. "Nonlinear vibration of hybrid laminated plates resting on elastic foundations in thermal environments", *Appl. Math. Model.*, **36**, pp. 6275-6290 (2012).
12. Shen, H.S. and Wang, H. "Nonlinear vibration of shear deformable FGM cylindrical panels resting on elastic foundations in thermal environments", *Compos. Part B Eng.*, **60**, pp. 167-177 (2014).
13. Huang, Z.Y., Lu, C.F., and Chen, W.Q. "Benchmark solutions for functionally graded thick plates resting on Winkler-Pasternak elastic foundations", *Compos. Struct.*, **85**, pp. 95-104 (2008).
14. Dehghan, M. and Baradaran, G.H. "Buckling and free vibration analysis of thick rectangular plates resting on elastic foundation using mixed finite element and differential quadrature method", *Appl. Math. Comput.*, **218**, pp. 2772-2784 (2011).
15. Qin, Q.H. and Diao, S. "Nonlinear analysis of thick plates on an elastic foundation by HT FE with p-extension capabilities", *Int. J. Solid Struct.*, **33**, pp. 4583-4604 (1996).
16. Duc, N.D. and Cong, P.H. "Nonlinear postbuckling of symmetric S-FGM plates resting on elastic foundations using higher order shear deformation plate theory in thermal environments", *Compos. Struct.*, **100**, pp. 566-574 (2013).
17. Fallah, A., Aghdam, M.M., and Kargarnovin, M.H. "Free vibration analysis of moderately thick functionally graded plates on elastic foundation using the extended Kantorovich method", *Arch. Appl. Mech.*, **83**, pp. 177-191 (2013).
18. Fallah, A. and Aghdam, M.M. "Nonlinear free vibration and post-buckling analysis of functionally graded beams on nonlinear elastic foundation", *Eur. J. Mech. A/Solids*, **30**, pp. 571-583 (2011).
19. Taczala, M., Buczkowski, R., and Kleiber, M. "Nonlinear free vibration of pre- and post-buckled FGM plates on two-parameter foundation in the thermal environment", *Compos. Struct.*, **137**, pp. 85-92 (2016).
20. Yang, Z., Yuan-yuan, G., and Fangin, B. "Solution for a rectangular plate on elastic foundation with free edges using reciprocal theorem method", *Math. Eterna.*, **2**, pp. 335-343 (2012).

21. Sundararajan, N., Prakash, T., and Ganapathi, M. "Nonlinear free flexural vibrations of functionally graded rectangular and skew plates under thermal environments", *Finite Elem. Anal. Des.*, **42**, pp. 152-168 (2005).
22. Huang, X.-L. and Shen, H.-S. "Nonlinear vibration and dynamic response of functionally graded plates in thermal environments", *Int. J. Solids Struct.*, **41**, pp. 2403-2427 (2004).
23. Civalek, Ö. "Nonlinear analysis of thin rectangular plates on Winkler-Pasternak elastic foundations by DSC-HDQ methods", *Appl. Math. Model.*, **31**, pp. 606-624 (2007).
24. Qin, Q.H. "Nonlinear analysis of reissner plates on an elastic foundation by the bem", *Int. J. Solids Struct.*, **30**, pp. 3101-3111 (1993).
25. Singha, M.K. and Daripa, R. "Nonlinear vibration and dynamic stability analysis of composite plates", *J. Sound Vib.*, **328**, pp. 541-554 (2009).
26. Thi, V.T.A. and Duc, N.D. "Nonlinear response of a shear deformable S- FGM shallow spherical shell with ceramic-metal- ceramic layers resting on an elastic foundation in a thermal environment", *Mech. Adv. Mater. Struct.*, **23**, pp. 926-934 (2016).
27. Tornabene, F., Fantuzzi, N., Viola, E., and Reddy, J.N. "Winkler-Pasternak foundation effect on the static and dynamic analyses of laminated doubly-curved and degenerate shells and panels", *Compos. Part B Eng.*, **57**, pp. 269-296 (2014).
28. Tornabene, F., Viola, E., and Fantuzzi, N. "General higher-order equivalent single layer theory for free vibrations of doubly-curved laminated composite shells and panels", *Compos. Struct.*, **104**, pp. 94-117 (2013).
29. Szekrenyes, A. "Nonsingular crack modelling in orthotropic plates by four equivalent single layers", *Eur. J. Mech. - A/Solids*, **55**, pp. 73-99 (2016a).
30. Szekrenyes, A. "Semi-layerwise analysis of laminated plates with nonsingular delamination-The theorem of autocontinuity", *Appl. Math. Model.*, **40**(2), pp. 1344-1371 (2016b).
31. Touloukin, Y.S., *Thermophysical Properties of High Temperature Solid Materials*, MacMillan, New York (1967).
32. Petyt, M., *Introduction to Finite Element Vibration Analysis*, Cambridge University Press, 2nd Ed. (2010).
33. Cook, R.D., Malkus, D.S., Plesha, M.E., and Witt, R.J., *Concepts and Applications of Finite Element Analysis*, Fourth Edn., John Wiley & Sons (Asia), Pvt. Ltd. (2002).

Appendix

$$c_1 = \frac{4}{3h^2}, \quad \beta_x = \frac{\partial w_0}{\partial x}, \quad \beta_y = \frac{\partial w_0}{\partial y}. \quad (\text{A.1})$$

Strain terms:

$$\begin{aligned} \{\varepsilon_L\} &= \begin{Bmatrix} \varepsilon_1^0 \\ \varepsilon_2^0 \\ \varepsilon_6^0 \\ \varepsilon_5^0 \\ \varepsilon_4^0 \end{Bmatrix} + z \begin{Bmatrix} \kappa_1^1 \\ \kappa_2^1 \\ \kappa_6^1 \\ \kappa_5^1 \\ \kappa_4^1 \end{Bmatrix} + z^2 \begin{Bmatrix} \kappa_1^2 \\ \kappa_2^2 \\ \kappa_6^2 \\ \kappa_5^2 \\ \kappa_4^2 \end{Bmatrix} + z^3 \begin{Bmatrix} \kappa_1^3 \\ \kappa_2^3 \\ \kappa_6^3 \\ \kappa_5^3 \\ \kappa_4^3 \end{Bmatrix}, \\ \{\varepsilon_{NL}\} &= \frac{1}{2} \begin{Bmatrix} \varepsilon_1^{nl0} \\ \varepsilon_2^{nl0} \\ \varepsilon_6^{nl0} \\ \varepsilon_5^{nl0} \\ \varepsilon_4^{nl0} \end{Bmatrix} + \frac{z}{2} \begin{Bmatrix} \kappa_1^{nl1} \\ \kappa_2^{nl1} \\ \kappa_6^{nl1} \\ \kappa_5^{nl1} \\ \kappa_4^{nl1} \end{Bmatrix} + \frac{z^2}{2} \begin{Bmatrix} \kappa_1^{nl2} \\ \kappa_2^{nl2} \\ \kappa_6^{nl2} \\ \kappa_5^{nl2} \\ \kappa_4^{nl2} \end{Bmatrix} \\ &\quad + \frac{z^3}{2} \begin{Bmatrix} \kappa_1^{nl3} \\ \kappa_2^{nl3} \\ \kappa_6^{nl3} \\ \kappa_5^{nl3} \\ \kappa_4^{nl3} \end{Bmatrix} + \frac{z^4}{2} \begin{Bmatrix} \kappa_1^{nl4} \\ \kappa_2^{nl4} \\ \kappa_6^{nl4} \\ \kappa_5^{nl4} \\ \kappa_4^{nl4} \end{Bmatrix} \\ &\quad + \frac{z^5}{2} \begin{Bmatrix} \kappa_1^{nl5} \\ \kappa_2^{nl5} \\ \kappa_6^{nl5} \\ \kappa_5^{nl5} \\ \kappa_4^{nl5} \end{Bmatrix} + \frac{z^6}{2} \begin{Bmatrix} \kappa_1^{nl6} \\ \kappa_2^{nl6} \\ \kappa_6^{nl6} \\ \kappa_5^{nl6} \\ \kappa_4^{nl6} \end{Bmatrix}. \quad (\text{A.2}) \end{aligned}$$

Linear strain terms:

$$\begin{aligned} \begin{Bmatrix} \varepsilon_1^0 \\ \varepsilon_2^0 \\ \varepsilon_6^0 \\ \varepsilon_5^0 \\ \varepsilon_4^0 \end{Bmatrix} &= \begin{Bmatrix} \frac{\partial u_0}{\partial x} \\ \frac{\partial v_0}{\partial y} \\ \frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \\ \theta_y + \frac{\partial w_0}{\partial x} \\ -\theta_x + \frac{\partial w_0}{\partial y} \end{Bmatrix}, \\ \begin{Bmatrix} \kappa_1^1 \\ \kappa_2^1 \\ \kappa_6^1 \\ \kappa_5^1 \\ \kappa_4^1 \end{Bmatrix} &= \begin{Bmatrix} \frac{\partial \theta_y}{\partial x} \\ -\frac{\partial \theta_x}{\partial y} \\ -\frac{\partial \theta_x}{\partial x} + \frac{\partial \theta_y}{\partial y} \\ 0 \\ 0 \end{Bmatrix}, \quad \begin{Bmatrix} \kappa_1^2 \\ \kappa_2^2 \\ \kappa_6^2 \\ \kappa_5^2 \\ \kappa_4^2 \end{Bmatrix} = c_2 \begin{Bmatrix} 0 \\ 0 \\ 0 \\ \theta_y + \beta_x \\ -\theta_x + \beta_y \end{Bmatrix}, \\ \begin{Bmatrix} \kappa_1^3 \\ \kappa_2^3 \\ \kappa_6^3 \\ \kappa_5^3 \\ \kappa_4^3 \end{Bmatrix} &= c_1 \begin{Bmatrix} -\frac{\partial \theta_y}{\partial x} - \frac{\partial \beta_x}{\partial x} \\ \frac{\partial \theta_x}{\partial y} - \frac{\partial \beta_y}{\partial y} \\ -\frac{\partial \theta_y}{\partial y} + \frac{\partial \theta_x}{\partial x} - \frac{\partial \beta_x}{\partial y} - \frac{\partial \beta_y}{\partial x} \\ 0 \\ 0 \end{Bmatrix}, \end{aligned}$$

$$c_2 = 3c_1. \quad (\text{A.2.1})$$

Non-linear strain terms:

$$\begin{aligned} \varepsilon_1^{nl0} &= [(u_{0,x})^2 + (v_{0,x})^2 + (w_{0,x})^2], \\ \varepsilon_2^{nl0} &= [(u_{0,y})^2 + (v_{0,y})^2 + (w_{0,y})^2], \\ \varepsilon_6^{nl0} &= [2u_{0,x}u_{0,y} + 2v_{0,x}v_{0,y} + 2w_{0,x}w_{0,y}], \\ \varepsilon_5^{nl0} &= [2\theta_y u_{0,x} - 2\theta_x v_{0,x}], \\ \varepsilon_4^{nl0} &= [2\theta_y u_{0,y} - 2\theta_x v_{0,y}], \\ \kappa_1^{nl1} &= [2u_{0,x}\theta_{y,x} - 2v_{0,x}\theta_{x,x}], \\ \kappa_2^{nl1} &= [2u_{0,y}\theta_{y,y} - 2v_{0,y}\theta_{x,y}], \\ \kappa_6^{nl1} &= [2u_{0,x}\theta_{y,y} + 2u_{0,y}\theta_{y,x} - 2v_{0,x}\theta_{x,y} - 2v_{0,y}\theta_{x,x}], \\ \kappa_5^{nl1} &= [2\theta_{y,x}\theta_y + 2\theta_{x,x}\theta_x], \\ \kappa_4^{nl1} &= [2\theta_{y,y}\theta_y + 2\theta_{x,y}\theta_x], \\ \kappa_1^{nl2} &= [(\theta_{y,x})^2 + (\theta_{x,x})^2], \\ \kappa_2^{nl2} &= [(\theta_{y,y})^2 + (\theta_{x,y})^2], \\ \kappa_6^{nl2} &= [2\theta_{y,x}\theta_{y,y} + 2\theta_{x,x}\theta_{x,y}], \\ \kappa_5^{nl2} &= [-6c_1\theta_y u_{0,x} - 6c_1\beta_x u_{0,x} + 6c_1\theta_x v_{0,x} \\ &\quad - 6c_1\beta_y v_{0,x}], \\ \kappa_4^{nl2} &= [-6c_1\theta_y u_{0,y} - 6c_1\beta_x u_{0,y} + 6c_1\theta_x v_{0,y} \\ &\quad - 6c_1\beta_y v_{0,y}], \\ \kappa_1^{nl3} &= [-2c_1u_{0,x}\theta_{y,x} - 2c_1u_{0,x}\beta_{x,x} + 2c_1v_{0,x}\theta_{x,x} \\ &\quad - 2c_1v_{0,x}\beta_{y,x}], \\ \kappa_2^{nl3} &= [-2c_1u_{0,y}\theta_{y,y} - 2c_1u_{0,y}\beta_{x,y} + 2c_1v_{0,y}\theta_{x,y} \\ &\quad - 2c_1v_{0,y}\beta_{y,y}], \\ \kappa_6^{nl3} &= [-2c_1u_{0,x}\theta_{y,y} - 2c_1u_{0,x}\beta_{x,y} - 2c_1u_{0,y}\theta_{y,x} \\ &\quad - 2c_1u_{0,y}\beta_{x,x} + 2c_1v_{0,x}\theta_{x,y} - 2c_1v_{0,x}\beta_{y,y} \\ &\quad + 2c_1v_{0,y}\theta_{x,x} - 2c_1v_{0,y}\beta_{y,x}], \\ \kappa_5^{nl3} &= [-8c_1\theta_{y,x}\theta_y - 8c_1\theta_{x,x}\theta_x - 2c_1\beta_{x,x}\theta_y \\ &\quad + 2c_1\beta_{y,x}\theta_x - 6c_1\theta_{y,x}\beta_x - 6c_1\theta_{x,x}\beta_y], \\ \kappa_4^{nl3} &= [-8c_1\theta_{y,y}\theta_y - 8c_1\theta_{x,y}\theta_x - 2c_1\beta_{x,y}\theta_y \\ &\quad + 2c_1\beta_{y,y}\theta_x - 6c_1\theta_{y,y}\beta_x + 6c_1\theta_{x,y}\beta_y], \end{aligned}$$

$$\begin{aligned} \kappa_1^{nl4} &= [-2c_1(\theta_{y,x})^2 - 2c_1(\theta_{x,x})^2 - 2c_1\theta_{y,x}\beta_{x,x} \\ &\quad + 2c_1\theta_{x,x}\beta_{y,x}], \\ \kappa_2^{nl4} &= [-2c_1(\theta_{y,y})^2 - 2c_1(\theta_{x,y})^2 - 2c_1\theta_{y,y}\beta_{x,y} \\ &\quad + 2c_1\theta_{x,y}\beta_{y,y}], \\ \kappa_6^{nl4} &= [-4c_1\theta_{y,y}\theta_{y,x} - 2c_1\theta_{y,y}\beta_{x,x} - 4\theta_{x,x}\theta_{x,y} \\ &\quad + 2c_1\theta_{x,x}\beta_{y,y} - 2c_1\beta_{x,y}\theta_{y,x} + 2c_1\theta_{x,y}\beta_{y,x}], \\ \kappa_5^{nl4} &= 0, \quad \kappa_4^{nl4} = 0, \quad \kappa_1^{nl5} = 0, \\ \kappa_2^{nl5} &= 0, \quad \kappa_6^{nl5} = 0, \\ \kappa_5^{nl5} &= [6c_1^2\theta_{x,x}\theta_x - 6c_1^2\beta_{y,x}\theta_x - 6c_1^2\theta_{x,x}\beta_y \\ &\quad + 6c_1^2\beta_{y,x}\beta_y + 6c_1^2\theta_{y,x}\theta_y + 6c_1^2\beta_{x,x}\theta_y \\ &\quad + 6c_1^2\theta_{y,x}\beta_x + 6c_1^2\beta_{x,x}\beta_x], \\ \kappa_4^{nl5} &= [6c_1^2\theta_{y,y}\theta_y + 6c_1^2\beta_{x,y}\theta_y + 6c_1^2\theta_{y,y}\beta_x \\ &\quad + 6c_1^2\beta_{x,y}\beta_x + 6c_1^2\theta_{x,y}\theta_x - 6c_1^2\beta_{y,y}\theta_x \\ &\quad - 6c_1^2\theta_{x,y}\beta_y + 6c_1^2\beta_{y,y}\beta_y], \\ \kappa_1^{nl6} &= [c_1^2(\theta_{y,x})^2 + c_1^2(\beta_{x,x})^2 + c_1^2(\theta_{x,x})^2 + c_1^2(\beta_{y,x})^2 \\ &\quad + 2c_1^2\theta_{y,x}\beta_{x,x} - 2c_1^2\theta_{x,x}\beta_{y,x}], \\ \kappa_2^{nl6} &= [c_1^2(\theta_{y,y})^2 + c_1^2(\beta_{x,y})^2 + c_1^2(\theta_{x,y})^2 + c_1^2(\beta_{y,y})^2 \\ &\quad + 2c_1^2\theta_{y,y}\beta_{x,y} - 2c_1^2\theta_{x,y}\beta_{y,y}], \\ \kappa_6^{nl6} &= [2c_1^2\theta_{y,x}\theta_{y,y} + 2c_1^2\theta_{y,x}\beta_{x,y} + 2c_1^2\beta_{x,x}\theta_{y,y} \\ &\quad + 2c_1^2\beta_{x,x}\beta_{x,y} + 2c_1^2\theta_{x,x}\theta_{x,y} - 2c_1^2\theta_{x,x}\beta_{y,y} \\ &\quad - 2c_1^2\beta_{y,x}\theta_{x,y} + 2c_1^2\beta_{y,x}\beta_{y,y}], \\ \kappa_5^{nl6} &= 0, \quad \kappa_4^{nl6} = 0. \end{aligned} \quad (\text{A.2.2})$$

The linear $[H_L]$ and nonlinear $[H_{NL}]$ thickness matrices are calculated by Eq. (A.2.3) as shown in Box I.

Inertia Matrix $[I]$:

$$\begin{aligned} [I]_{1,1} &= I_0, \quad [I]_{2,2} = I_0, \quad [I]_{3,3} = I_0, \\ [I]_{4,4} &= c_1^2 \times I_6, \quad [I]_{4,7} = -c_1 \times J_4, \\ [I]_{5,5} &= c_1^2 \times I_6, \quad [I]_{5,6} = c_1 \times J_4, \\ [I]_{6,5} &= c_1 \times J_4, \quad [I]_{6,6} = K_2, \\ [I]_{7,4} &= -c_1 \times J_4, \quad [I]_{7,7} = K_2, \end{aligned} \quad (\text{A.3})$$

$$\begin{aligned}
H_L &= \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & z & 0 & 0 & 0 & 0 & z^2 & 0 & 0 & 0 & 0 & z^3 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & z & 0 & 0 & 0 & 0 & z^2 & 0 & 0 & 0 & 0 & z^3 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & z & 0 & 0 & 0 & 0 & z^2 & 0 & 0 & 0 & 0 & z^3 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & z & 0 & 0 & 0 & 0 & z^2 & 0 & 0 & 0 & 0 & z^3 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & z & 0 & 0 & 0 & 0 & z^2 & 0 & 0 & 0 & 0 & z^3 \end{bmatrix}, \\
H_{NL} &= \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & z & 0 & 0 & 0 & 0 & z^2 & 0 & 0 & 0 & 0 & z^3 & 0 & 0 & 0 & 0 & z^4 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & z & 0 & 0 & 0 & 0 & z^2 & 0 & 0 & 0 & 0 & z^3 & 0 & 0 & 0 & 0 & z^4 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & z & 0 & 0 & 0 & 0 & z^2 & 0 & 0 & 0 & 0 & z^3 & 0 & 0 & 0 & 0 & z^4 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & z & 0 & 0 & 0 & 0 & z^2 & 0 & 0 & 0 & 0 & z^3 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 & z & 0 & 0 & 0 & 0 & z^2 & 0 & 0 & 0 & 0 & z^3 & 0 & 0 & 0 \end{bmatrix} \\
&\quad \begin{bmatrix} 0 & 0 & z^5 & 0 & 0 & 0 & 0 & z^6 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & z^5 & 0 & 0 & 0 & 0 & z^6 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & z^5 & 0 & 0 & 0 & 0 & z^6 & 0 & 0 \\ z^4 & 0 & 0 & 0 & 0 & z^5 & 0 & 0 & 0 & 0 & z^6 & 0 \\ 0 & z^4 & 0 & 0 & 0 & 0 & z^5 & 0 & 0 & 0 & 0 & z^6 \end{bmatrix}. \tag{A.2.3}
\end{aligned}$$

Box I

where:

$$I_i = \int_{-h/2}^{h/2} \rho z^i dz, \quad (i = 0, 2, 4, 6),$$

$$c_1 = \frac{4}{3h^2}, \quad K_2 = I_2 - 2c_1 J_4 + c_1^2 I_6,$$

$$J_4 = I_4 - c_1 I_6.$$

The work done due to nonlinear temperature rise $W_{\Delta T}$:

$$W_{\Delta T} = \int_{-h/2}^{h/2} \{\varepsilon_G\}^{tr} [S] \{\varepsilon_G\}, \tag{A.4}$$

where:

$$\{\varepsilon_G\} = \frac{1}{2} \begin{bmatrix} \{u_{,x}^2 + v_{,x}^2 + w_{,x}^2\}, \\ \{u_{,y}^2 + v_{,y}^2 + w_{,y}^2\}, \\ 2\{u_{,x}u_{,y} + v_{,x}v_{,y} + w_{,x}w_{,y}\} \end{bmatrix},$$

or:

$$\{\varepsilon_G\} = [H] [A_g] [\mathbb{N}] = [B_G] \{\delta\},$$

$$[H] = \begin{bmatrix} 1 & 0 & 0 & -c_1 z^3 & 0 & 0 & z - c_1 z^3 \\ 0 & 1 & 0 & 0 & -c_1 z^3 & z - c_1 z^3 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 \end{bmatrix},$$

$$[A_g] = \begin{bmatrix} \{u_{,x} + v_{,x} + w_{,x}\} \\ \{u_{,y} + v_{,y} + w_{,y}\} \\ \{u_{,x}u_{,y} + v_{,x}v_{,y} + w_{,x}w_{,y}\} \end{bmatrix}, \quad [\mathbb{N}] = \begin{bmatrix} u_{,x} \\ u_{,y} \\ v_{,x} \\ v_{,y} \\ w_{,x} \\ w_{,y} \end{bmatrix},$$

$$[S] = \begin{bmatrix} N_x^T & N_{xy}^T & 0 & 0 & 0 & 0 \\ N_{xy}^T & N_y^T & 0 & 0 & 0 & 0 \\ 0 & 0 & N_x^T & N_{xy}^T & 0 & 0 \\ 0 & 0 & N_{xy}^T & N_y^T & 0 & 0 \\ 0 & 0 & 0 & 0 & N_x^T & N_{xy}^T \\ 0 & 0 & 0 & 0 & N_{xy}^T & N_y^T \end{bmatrix}.$$

Terms of $[A_g]$ matrix:

$$[A_g]_{1,1} = u_{,x}, \quad [A_g]_{1,3} = v_{,x}, \quad [A_g]_{1,5} = w_{,x},$$

$$[A_g]_{2,2} = u_{,y}, \quad [A_g]_{2,4} = v_{,y}, \quad [A_g]_{2,6} = w_{,y},$$

$$[A_g]_{3,1} = u_{,y}, \quad [A_g]_{3,2} = u_{,x}, \quad [A_g]_{3,3} = v_{,y},$$

$$[A_g]_{3,4} = v_{,x}, \quad [A_g]_{3,5} = w_{,y}, \quad [A_g]_{3,6} = w_{,x},$$

$$[A_g]_{4,1} = \theta_y, \quad [A_g]_{4,3} = -\theta_x, \quad [A_g]_{4,17} = v_{,x},$$

$$[A_g]_{4,18} = u_{,x}, \quad [A_g]_{5,2} = \theta_y, \quad [A_g]_{5,4} = -\theta_x,$$

$$[A_g]_{5,17} = -v_{,y}, \quad [A_g]_{5,18} = u_{,y},$$

$$[A_g]_{6,1} = \theta_{y,x}, \quad [A_g]_{6,3} = -\theta_{x,x},$$

$$[A_g]_{6,11} = -v_{,x}, \quad [A_g]_{6,13} = u_{,x},$$

$$[A_g]_{7,2} = \theta_{y,y}, \quad [A_g]_{7,4} = -\theta_{x,y},$$

$$[A_g]_{7,12} = -v_{,y}, \quad [A_g]_{7,14} = u_{,y},$$

$$[A_g]_{8,1} = \theta_{y,y}, \quad [A_g]_{8,2} = \theta_{y,x},$$

$$\begin{aligned}
[A_g]_{8,3} &= -\theta_{x,y}, & [A_g]_{8,4} &= -\theta_{x,x}, & [A_g]_{17,12} &= -c_1 \times v_y, & [A_g]_{17,14} &= -c_2 \times u_y, \\
[A_g]_{8,11} &= -v_y, & [A_g]_{8,12} &= -v_x, & [A_g]_{18,1} &= -c_1 \times \theta_{y,y} - c_1 \times \beta_{x,y}, \\
[A_g]_{8,13} &= u_y, & [A_g]_{8,14} &= u_x, & [A_g]_{18,2} &= -c_1 \times \theta_{y,x} - c_1 \times \beta_{x,x}, \\
[A_g]_{9,11} &= \theta_x, & [A_g]_{9,13} &= \theta_y, & [A_g]_{18,3} &= c_1 \times \theta_{x,y} - c_1 \times \beta_{y,y}, \\
[A_g]_{9,17} &= \theta_{x,x}, & [A_g]_{9,18} &= \theta_{y,x}, & [A_g]_{18,4} &= c_1 \times \theta_{x,x} - c_1 \times \beta_{y,x}, \\
[A_g]_{10,12} &= \theta_x, & [A_g]_{10,14} &= \theta_y, & [A_g]_{18,7} &= -c_1 \times u_y, & [A_g]_{18,8} &= -c_1 \times u_x, \\
[A_g]_{10,17} &= \theta_{x,y}, & [A_g]_{10,18} &= \theta_{y,y}, & [A_g]_{18,9} &= -c_1 \times v_y, & [A_g]_{18,10} &= -c_1 \times v_x, \\
[A_g]_{11,11} &= \theta_{x,x}, & [A_g]_{11,13} &= \theta_{y,x}, & [A_g]_{18,11} &= c_1 \times v_y, & [A_g]_{18,12} &= c_1 \times v_x, \\
[A]_{12,12} &= \theta_{x,y}, & [A]_{12,14} &= \theta_{y,y}, & [A_g]_{18,13} &= -c_1 \times u_y, & [A_g]_{18,14} &= -c_1 \times u_x, \\
[A]_{13,11} &= \theta_{x,y}, & [A]_{13,12} &= \theta_{x,x}, & [A_g]_{19,7} &= -c_1 \times \theta_y, & [A_g]_{19,9} &= c_1 \times \theta_x, \\
[A]_{13,13} &= \theta_{y,y}, & [A_g]_{13,14} &= \theta_{y,x}, & [A_g]_{19,11} &= -4 \times c_1 \times \theta_x - c_2 \times \beta_y, \\
[A_g]_{14,1} &= -c_2 \times \theta_y - c_2 \times \beta_x, & & & [A_g]_{19,13} &= -4 \times c_1 \times \theta_y - c_2 \times \beta_x, \\
[A]_{14,3} &= c_2 \times \theta_x - c_2 \times \beta_y, & & & [A_g]_{19,15} &= -c_2 \times \theta_{y,x}, & [A_g]_{19,16} &= -c_2 \times \theta_{x,x}, \\
[A]_{14,15} &= -c_2 \times u_x, & [A]_{14,16} &= -c_2 \times v_x, & [A_g]_{19,17} &= -4 \times c_1 \times \theta_{x,x} + c_1 \times \beta_{y,x}, \\
[A_g]_{14,17} &= c_2 \times v_x, & [A_g]_{14,18} &= -c_2 \times u_x, & [A_g]_{19,18} &= -4 \times c_1 \times \theta_{y,x} - c_1 \times \beta_{x,x}, \\
[A]_{15,2} &= -c_2 \times \theta_y - c_2 \times \beta_x, & & & [A_g]_{20,8} &= -c_1 \times \theta_y, & [A_g]_{20,10} &= c_1 \times \theta_x, \\
[A]_{15,4} &= c_2 \times \theta_x - c_2 \times \beta_y, & & & [A_g]_{20,12} &= -4 \times c_1 \times \theta_x + c_2 \times \beta_y, \\
[A]_{15,15} &= -c_2 \times u_y, & [A_g]_{15,16} &= -c_2 \times v_y, & [A_g]_{20,14} &= -4 \times c_1 \times \theta_y - c_2 \times \beta_x, \\
[A_g]_{15,17} &= c_2 \times v_y, & [A]_{15,18} &= -c_2 \times u_y, & [A_g]_{20,15} &= -c_2 \times \theta_{y,y}, & [A_g]_{20,16} &= c_2 \times \theta_{x,y}, \\
[A]_{16,1} &= -c_1 \times \beta_{x,x} - c_1 \times \theta_{y,x}, & & & [A_g]_{20,17} &= -4 \times c_1 \times \theta_{x,y} + c_1 \times \beta_{y,y}, \\
[A_g]_{16,3} &= c_1 \times \theta_{x,x} - c_1 \times \beta_{y,x}, & & & [A_g]_{20,18} &= -4 \times c_1 \times \theta_{y,y} - c_1 \times \beta_{x,y}, \\
[A_g]_{16,7} &= -c_1 \times u_x, & [A_g]_{16,9} &= c_1 \times v_x, & [A_g]_{21,7} &= -c_1 \times \theta_{y,x}, & [A_g]_{21,9} &= c_1 \times \theta_{x,x}, \\
[A_g]_{16,11} &= c_1 \times v_x, & [A_g]_{16,13} &= -c_1 \times u_x, & [A_g]_{21,11} &= c_1 \times \beta_{y,x} - 2c_1 \times \theta_{x,x}, \\
[A_g]_{17,2} &= -c_1 \times \theta_{y,y} - c_1 \times \beta_{x,y}, & & & [A_g]_{21,13} &= -c_1 \times \beta_{x,x} - 2c_1 \times \theta_{y,x},
\end{aligned}$$

$$\begin{aligned}
[A_g]_{22,8} &= -c_1 \times \theta_{y,y}, & [A_g]_{22,10} &= c_1 \times \theta_{x,y}, & [A_g]_{32,10} &= c_1^2 \times \beta_{y,y} - c_1^2 \times \theta_{x,y}, \\
[A_g]_{22,12} &= -2c_1 \times \theta_{x,y} + c_1 \times \beta_{y,y}, & & & [A_g]_{32,12} &= -c_1^2 \times \beta_{y,y} + c_1^2 \times \theta_{x,y}, \\
[A_g]_{22,14} &= -2c_1 \times \theta_{y,y} - c_1 \times \beta_{x,y}, & & & [A_g]_{32,14} &= c_1^2 \times \beta_{x,y} + c_1^2 \times \theta_{y,y}, \\
[A_g]_{23,7} &= -c_1 \times \theta_{y,y}, & [A_g]_{23,8} &= -c_1 \times \theta_{y,x}, & [A_g]_{33,7} &= c_1^2 \times \beta_{x,y} + c_1^2 \times \theta_{y,y}, \\
[A_g]_{23,9} &= c_1 \times \theta_{x,y}, & [A_g]_{23,10} &= c_1 \times \theta_{x,x}, & [A_g]_{33,8} &= c_1^2 \times \beta_{x,x} + c_1^2 \times \theta_{y,y}, \\
[A_g]_{23,11} &= -2c_1 \times \theta_{x,y} + c_1 \times \beta_{y,y}, & & & [A_g]_{33,9} &= c_1^2 \times \beta_{y,y} - c_1^2 \times \theta_{y,x}, \\
[A_g]_{23,12} &= -2c_1 \times \theta_{x,x} + c_1 \times \beta_{y,x}, & & & [A_g]_{33,10} &= c_1^2 \times \beta_{y,x} - c_1^2 \times \theta_{x,x}, \\
[A_g]_{23,13} &= -2c_1 \times \theta_{y,y} - c_1 \times \beta_{x,y}, & & & [A_g]_{33,11} &= -c_1^2 \times \beta_{y,y} + c_1^2 \times \theta_{x,y}, \\
[A_g]_{23,14} &= -2c_1 \times \theta_{y,x} - c_1 \times \beta_{x,x}, & & & [A_g]_{33,12} &= -c_1^2 \times \beta_{y,x} + c_1^2 \times \theta_{x,x}, \\
[A_g]_{29,7} &= c_2 c_1 \times \theta_y + c_2 c_1 \times \beta_x, & & & [A_g]_{33,14} &= c_1^2 \times \beta_{x,y} + c_1^2 \times \theta_{y,y}. \tag{A.5} \\
[A_g]_{29,9} &= -c_2 c_1 \times \theta_x + c_2 c_1 \times \beta_y, \\
[A_g]_{29,11} &= c_2 c_1 \times \theta_x - c_2 c_1 \times \beta_y, \\
[A_g]_{29,13} &= c_2 c_1 \times \theta_y + c_2 c_1 \times \beta_x, \\
[A_g]_{29,15} &= c_2 c_1 \times \theta_{y,x} + c_2 c_1 \times \beta_{x,x}, \\
[A_g]_{29,16} &= -c_2 c_1 \times \theta_{x,x} + c_2 c_1 \times \beta_{y,x}, \\
[A_g]_{29,17} &= c_2 c_1 \times \theta_{x,x} - c_2 c_1 \times \beta_{y,x}, \\
[A_g]_{29,18} &= c_2 c_1 \times \theta_{y,x} + c_2 c_1 \times \beta_{x,x}, \\
[A_g]_{30,8} &= c_2 c_1 \times \theta_y + c_2 c_1 \times \beta_x, \\
[A_g]_{30,10} &= -c_2 c_1 \times \theta_x + c_2 c_1 \times \beta_y, \\
[A_g]_{30,12} &= c_2 c_1 \times \theta_x - c_2 c_1 \times \beta_y, \\
[A_g]_{30,14} &= c_2 c_1 \times \theta_y + c_2 c_1 \times \beta_x, \\
[A_g]_{30,15} &= c_2 c_1 \times \theta_{y,y} + c_2 c_1 \times \beta_{x,y}, \\
[A_g]_{30,16} &= -c_2 c_1 \times \theta_{x,y} + c_2 c_1 \times \beta_{y,y}, \\
[A_g]_{30,17} &= c_2 c_1 \times \theta_{x,y} - c_2 c_1 \times \beta_{y,y}, \\
[A_g]_{30,18} &= c_2 c_1 \times \theta_{y,y} + c_2 c_1 \times \beta_{x,y}, \\
[A_g]_{31,7} &= c_1^2 \times \beta_{x,x} + c_1^2 \times \theta_{y,x}, \\
[A_g]_{31,9} &= c_1^2 \times \beta_{y,x} - c_1^2 \times \theta_{x,x}, \\
[A_g]_{31,11} &= -c_1^2 \times \beta_{y,x} + c_1^2 \times \theta_{x,x}, \\
[A_g]_{31,13} &= c_1^2 \times \beta_{x,x} + c_1^2 \times \theta_{y,x}, \\
[A_g]_{32,8} &= c_1^2 \times \beta_{x,y} + c_1^2 \times \theta_{y,y},
\end{aligned}$$

Terms of $[B_G]$ matrix:

$$\begin{aligned}
[B_G]_{1,1} &= \partial/\partial x, & [B_G]_{2,2} &= \partial/\partial y, \\
[B_G]_{3,1} &= \partial/\partial y, & [B_G]_{3,2} &= \partial/\partial x, \\
[B_G]_{4,3} &= \partial/\partial x, & [B_G]_{4,7} &= 1, \\
[B_G]_{5,3} &= \partial/\partial y, & [B_G]_{5,6} &= -1, \\
[B_G]_{6,7} &= \partial/\partial x, & [B_G]_{7,6} &= -\partial/\partial y, \\
[B_G]_{8,6} &= -\partial/\partial x, & [B_G]_{8,7} &= \partial/\partial y, \\
[B_G]_{14,4} &= -c_2, & [B_G]_{14,7} &= -c_2, \\
[B_G]_{15,5} &= -c_2, & [B_G]_{15,6} &= c_2, \\
[B_G]_{16,4} &= -c_1 \times \partial/\partial x, & [B_G]_{16,7} &= -c_1 \times \partial/\partial x, \\
[B_G]_{17,5} &= -c_1 \times \partial/\partial y, & [B_G]_{17,6} &= c_1 \times \partial/\partial y, \\
[B_G]_{18,4} &= -c_1 \times \partial/\partial y, & [B_G]_{18,5} &= -c_1 \times \partial/\partial x, \\
[B_G]_{18,6} &= c_1 \times \partial/\partial x, & [B_G]_{18,7} &= -c_1 \times \partial/\partial y. \tag{A.6}
\end{aligned}$$

Terms of $[\mathbb{N}]$ matrix:

$$\begin{aligned}
[\mathbb{N}]_{1,1} &= \partial/\partial x, & [\mathbb{N}]_{2,1} &= \partial/\partial y, \\
[\mathbb{N}]_{3,2} &= \partial/\partial x, & [\mathbb{N}]_{4,2} &= \partial/\partial y, \\
[\mathbb{N}]_{5,3} &= \partial/\partial x, & [\mathbb{N}]_{6,3} &= \partial/\partial y, \\
[\mathbb{N}]_{7,4} &= \partial/\partial x, & [\mathbb{N}]_{8,4} &= \partial/\partial y, \\
[\mathbb{N}]_{9,5} &= \partial/\partial x, & [\mathbb{N}]_{10,5} &= \partial/\partial y, \\
[\mathbb{N}]_{11,6} &= \partial/\partial x, & [\mathbb{N}]_{12,6} &= \partial/\partial y,
\end{aligned}$$

$$\begin{aligned}
[\aleph]_{13,7} &= \partial/\partial x, & [\aleph]_{14,7} &= \partial/\partial y, \\
[\aleph]_{15,4} &= 1, & [\aleph]_{16,5} &= 1, \\
[\aleph]_{17,6} &= 1, & [\aleph]_{18,7} &= 1.
\end{aligned} \tag{A.7}$$

Biographies

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