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Research Note

## Applicability of a numerical model to predict distribution of suspended sediment concentration in Dithmarschen Bight

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### KEYWORDS

Suspended;  
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Model;  
Critical;  
Shear;  
Stress;  
Tide.

**Abstract.** The main concern of this investigation is to evaluate the ability of the Delft3D-flow package in studying the distribution of Suspended Sediment Concentration (SSC) over the depth in the Dithmarschen Bight. The area consists of tidal channels and tidal flats, with a prevailing semi-diurnal tide, and is tidally dominated. Required field data were prepared using the data collected by a transmissometer and a mechanical sampler. A factor of two of the measured SSC was used to evaluate the performance of the model, and some dissimilarity was found between the modeled and measured SSC. To verify the reason, two comparing procedures were carried out. First, evolutions of the vertical profile of the SSC from the model and the field were prepared and compared. In another procedure, snapshots of the distribution of SSC during different phases of a tidal cycle were prepared for both model results and field data. It was found that the predicted SSC values are in good agreement with field data during the periods of flood phase and low slack water. However, spatial dissimilarities are observed during the periods of high slack water and the ebb phase. An insufficient supply of sediment from the tidal flat predicted by the model was considered to be responsible.

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### 1. Introduction

Coastal zones have a high potential for numerous physical activities and are of critical economic importance. They encompass immense environmental, social and economic value, and, therefore, should be managed ecologically, ethically and economically. To achieve this aim, it requires a thorough understanding of the physical, chemical, biological and other processes involved. Among the physical processes is sediment transport.

Observations and field measurements are necessary but insufficient to describe these processes pre-

cisely, because of the size and nature of the area involved. The choices are numerical and computational techniques. These models involve the simulation of flow and sediment transport conditions based on the formulation and solution of mathematical relationships. When these models and the relevant computational techniques are established, they can be improved and refined as more data and additional or refined parameters become available. The task is to improve our understanding of their limitations and constraints, as well as knowledge of the physical processes involved.

The application of models in coastal engineering is reasonably advanced in terms of the prediction of flow hydrodynamics, but it is imprecise in the prediction of sediment transport and morphodynamics. The lack of sufficient and adequate field data, on the one hand,

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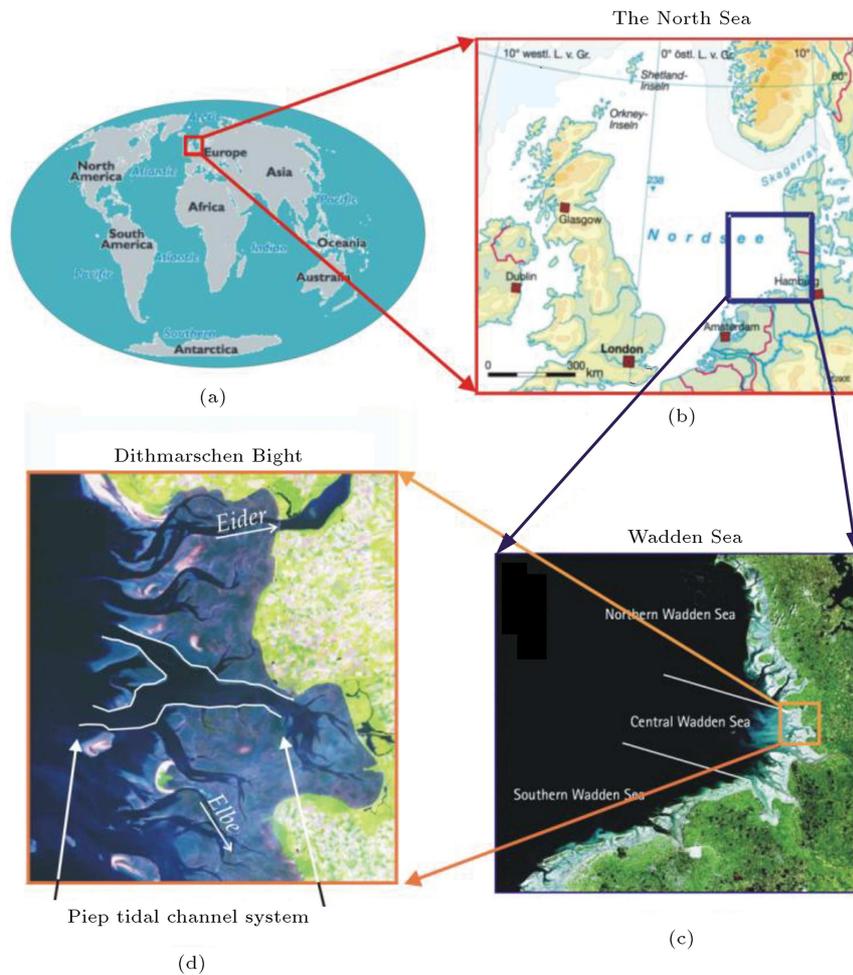


Figure 1. Geographical location of central Dithmarschen Bight.

and the lack of universally accepted equations and parameters, on the other, make prediction of sediment transport a challenging topic [1]. The main purpose of this study is to evaluate the predictive ability of a model developed using the Delft3D package to predict sediment dynamics for the coastal zone of Central Dithmarschen Bight, which is located on the German North Sea, as shown in Figure 1. The 3D flow model, incorporated with the sediment module of the package, was used for this investigation. The model results were compared with field data to investigate the performance of the model, with the main interest in the prediction of SSC, in order to find the reason or reasons for the weak correlation existing between model results and field data.

## 2. Area under investigation

Dithmarschen Bight is located between the Eider and Elbe estuaries and situated about 100 km north of Hamburg (Figure 1(c)). It consists of tidal channels, tidal flats and sand banks. It is tidally dominated and known as a well-mixed body of water with a

mean tidal range of about 3.2 m. The most dominant morphological features of the area are tidal flats, tidal channels and sand banks over the outer region. Under moderate conditions, the maximum mean water depth in the tidal channels is about 18m, and approximately 50% of the domain falls dry at low tide. The tidal flats and sandbanks are exposed at low water [2]. The most dynamic morphological units are found at the western boundary of the tidal flats, where wave action interferes with strong tidal and wind driven currents [3]. The banks and shoals in this region exhibit the highest migration rates. The sediments in suspension are mainly cohesive, consisting of very fine to medium-grain silt [4].

The specific area under investigation in this research is the Piep tidal channel system, which is part of Dithmarschen Bight, and is shown in Figure 1(d). It consists of two main channels, namely, Norderpiep and Süderpiep. These two channels conjunct together to form the Piep channel near the land on the tidal flat. The width of the channels and their rivulets (ending the tidal flats or the shore) varies spatially and temporally from a few meters to about 4 km.

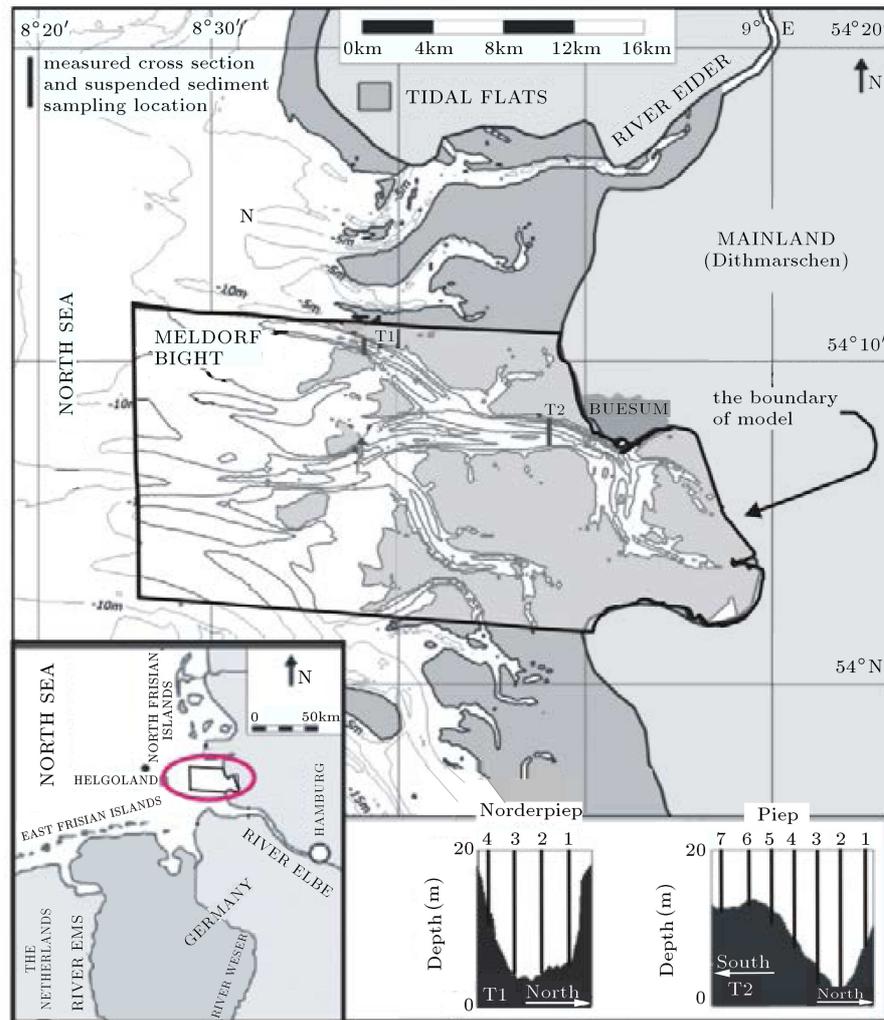


Figure 2. Locations of the cross sections in investigation and the position of monitoring points.

### 3. Material and methods

To obtain reliable results from the models, a comprehensive knowledge of the processes involved is necessary but insufficient. Precise values of parameters and variables derived on the basis of adequate field measurements are also needed to establish the model and, also, for the purpose of model calibration and validation.

The required field data for this study were collected from “Prediction of Medium Term Coastal Morphodynamics”, known as the PROMORPH project. It was executed during the period May 1999 to June 2002. The data were collected using equipped cruising vessels under different conditions.

The field data used in this study cover two cross sections in the two channels: Norderpiep (T1), and Piep (T2) (see Figure 2). Each time, the cruises were carried out across cross sections T1 and T2 for one full tidal cycle. In each measuring campaign, current velocity and SSC were collected at several stations

across the width of each cross section and at various depths of each station.

As seen in Figure 2, cross section T1 is in the northern channel, and it is quite narrow with a width of 770 meters and a depth of 2.8 to 16.1 meters. Cross section T2 is in the Piep channel, which is ended over the tidal flat area. The width of this cross section is about 1200 meters and its depth is between 6.2 and 17.9 meters. The date of the surveys, corresponding tidal range, period of measuring cruises, and the number of stations where the measurements were carried out, are summarized in Table 1. The boundaries of the model have been chosen far from the area of interest, namely, the Piep tidal system. This has ensured that the boundary conditions will not affect the hydrodynamics and sediment dynamics at the monitoring points. This area is bordered by a black curve in Figure 2. The model consists of a closed land boundary in the east and three open boundaries in the north, west, and south. For the open boundary, input data in terms of water levels were considered. It was a decision made due

**Table 1.** Number of ADCP transects, transmissometer profiles and water samples collected during measurement surveys at cross sections T1 and T2 under the PROMORPH project.

Channel	Date of measurement	Tidal range (m)	No. of stations	Measuring duration	No. of ADCP transect	No. of trans. profiles	No. of water samples
Cross section in Norderpiep channel (T1)	March 22nd 2000	4.0	4	From: 8:04 To: 20:02	65	126	5
	June 5th 2000	3.8	4	From: 9:23 to: 20:59	61	32	14
	September 5th 2000	3.1	4	From: 4:13 to: 16:57	59	105	28
	September 12th 2000	3.2	4	From: 4:59 to: 17:09	67	131	8
	December 5th 2000	2.3	4	From: 5:57 to: 18:08	73	44	11
Cross section in piep channel (T2)	March 23rd 2000	4.2	7	From: 7:33 to: 21:14	50	163	7
	June 6th 2000	3.7	7	From: 7:36 to: 16:24	37	90	15
	September 6th 2000	2.5	7	From: 4:32 to: 15:50	27	91	24
	September 13th 2000	3.4	7	From: 5:32 to:10:38	17	55	5
	December 6th 2000	2.4	7	From: 6:40 to:19:11	44	130	8

**Table 2.** Properties of the mud fraction.

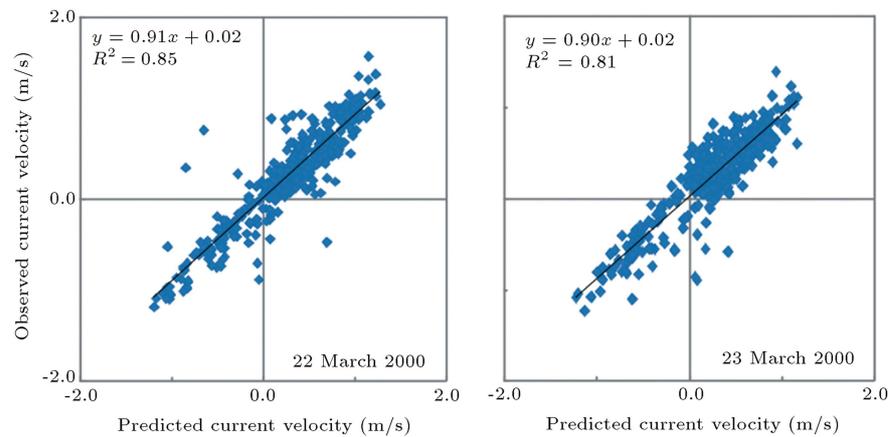
Period	Tidal range (m)	No. of measuring stations	Settling velocity (mm/s)	Critical bed shear stress for sedimentation ( $\text{N/m}^2$ )	Critical bed shear stress for erosion ( $\text{N/m}^2$ )	Erosion parameter ( $\text{kg/m}^2/\text{s}$ )
Mar. 21-23, 2000	4.0	20	1.60	2.88	0.89	5.10e-004
June 5-6, 2000	3.7	23	2.00	3.24	1.00	4.70e-004
Sept. 5-6, 2000	3.2	23	1.60	3.02	0.79	5.70e-004
Sept. 12-13, 2000	3.0	21	1.76	3.12	0.88	5.20e-004
Dec. 5-6, 2000	2.3	20	1.30	2.90	0.65	1.57e-004

to the availability of long time data collection at the site.

The grain size map of the area was developed by Escobar (2007) [5]. He carried out intensive experiments and determined a functional relationship between flow characteristic and grain size distribution. Regarding the sediment properties, altogether, five sediment fractions were used, of which, four describe

the non-cohesive sediments and one represents the mud fraction. The mud content and properties of the non-cohesive sediment fraction were those derived from sediment samples taken at several locations, as reported by Poerbandono and Mayerle (2005) [4]. Characteristics of the cohesive fractions, accounting for 75% of the sediment mixture, are listed in Table 2.

According to Rahbani (2011) [6], the effect of



**Figure 3.** Measured versus predicted current velocity for cross sections T1 (left side) and T2 (right side).

waves under moderate winds, having velocities less than 11 m/s, is ignorable. In the simulations, therefore, considering moderate conditions during all the campaigns, the effect of wind induced waves was withdrawn.

The hydrodynamic of the model was calibrated and validated by Palacio et al. (2005) [7], using collected ADCP data. They reported the mean absolute error of less than 0.2 m/s between computed and observed velocities at various cross-sections in the tidal channels. They also claimed that this value represents less than 20% of the tidally-averaged value, which can be considered an acceptable result for the hydrodynamic model. To show the relativity between the modeled and measured current velocity, Figure 3 is prepared. Measured velocities are presented on the vertical axis and modeled velocities are presented on the horizontal axis. The results for cross sections, T1 and T2, are presented in the left and right side, accordingly. The trend line for each graph is derived. Its equation and  $R$ -squared values are presented at the top of each graph. It can be seen that over 80% of the predicted current velocities are in good agreement with the measured ones.

To obtain SSC during the cruises of the PROMORPH project, the Niskin bottle, as a trap sampler, and the transmissometer, as an optical device, were employed. For collecting the SSC at different levels along the depth, the transmissometer, together with one CTD (Conductivity, Temperature and Depth) device, and one Niskin bottle sampler, were mounted on a frame. In each cruise, the frame was lowered at specified positions from the surface to near the bottom across each cross section. The CTD device in the frame provided the height at which the beam scatter data and samples are collected.

To convert the optical transmission data to SSC, the method described by Ohm (1985) [8] and Ricklefs (1989) [9] was employed. That is, those SSC determined from about 200 Niskin bottle samples were

plotted against the amount of transmission of light to derive a relationship. Through statistical analyses, Poerbandono (2003) [10] proposed Eq. (1) for this conversion:

$$c = (7A + 33)10^{-3}, \quad (1)$$

where,  $c$  is concentration of sediment, and  $A = -L^{-1} \ln(I)$  is the attenuation coefficient in which  $L$  is the transmissometer path length in cm, and  $I$  is the optical transmission as a decimal fraction.

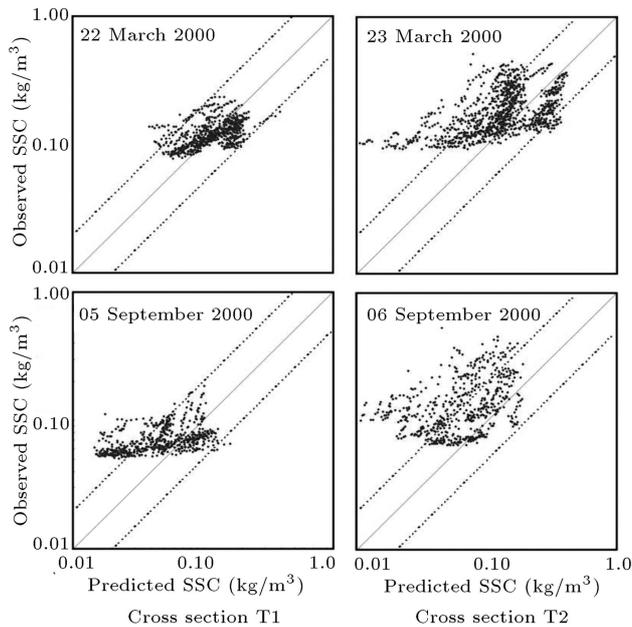
Using the RMAE method, Poerbandono (2003) [10] reported that the accuracy of optical measurements was about 30% on the basis of agreement with concentrations determined from physical samples. This low accuracy has been claimed to be due to the insufficient sensitivity of the optical beam transmissometer with a path length of only 2 cm for detecting low concentrations [11].

These data made it possible to evaluate the results of the 3D model at every spatial position and temporal situation of the area under investigation. That is, the model was provided with the monitoring points at the same geographical positions of the points where measurements were carried out; monitoring points 1 to 4 at cross section T1, and 1 to 7 at cross section T2, in Figure 2. The model was executed for the same period as the measuring campaigns were carried out. The SSC data derived from the field measurements were compared with those derived from the model.

## 4. Results and discussion

### 4.1. Evolution of the vertical profiles of suspended sediment concentration

As a first approach to verify the high quality of the model results, the SSC derived from the model were plotted against those derived from measurements. Figure 4 shows the scatter plot of the measured versus computed SSC, covering all simulated conditions for two cross sections, T1 and T2, during the neap and



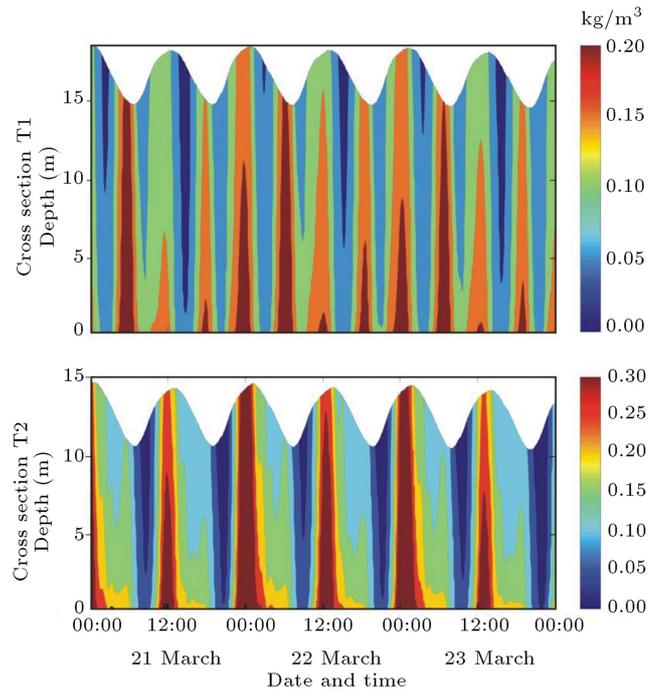
**Figure 4.** SSC predicted by model versus SSC derived from the field for cross-sections T1 and T2.

spring tide. The black dashed lines in the plots represent a factor of 2 and the central solid line represents perfect agreement between the measured and predicted suspended sediment concentrations.

It can be seen that for the entire two periods of measurements, and at both cross sections, a high proportion of values are within a factor of two, which, according to Kleinhans and Van Rijn (2002) [12], is considered quite high quality for predicting SSC. However, in cross section T2, a considerable number of points are also located outside the factor of two, which shows relatively poor correlation between the model and field data. The plots for other data sets mentioned in Table 1 showed the same pattern in both cross sections (see Rahbani (2011) [6]).

Two aspects that may affect the predictive ability of the sediment transport model, including: (a) the assessment of the effect of conditions specified along the open sea boundaries of the flow mode, and (b) the need to account for waves in the sediment transport simulations, were studied. Neither of these factors was found to be responsible (see Rahbani (2011) [6]).

To verify the cause of dissimilarity observed at cross section T2, two comparing procedures were carried out. First, evolutions of the vertical profile of the SSC from the model for a monitoring point at the middle of each cross section were prepared and compared (Figure 5). Two successive high concentrations of suspended sediment, due to the flood and ebb phase, can be seen for cross section T1, which is usual when taking into account the semidiurnal nature of the area. However, this is not the case for cross section T2. According to the model results in cross section T2, a

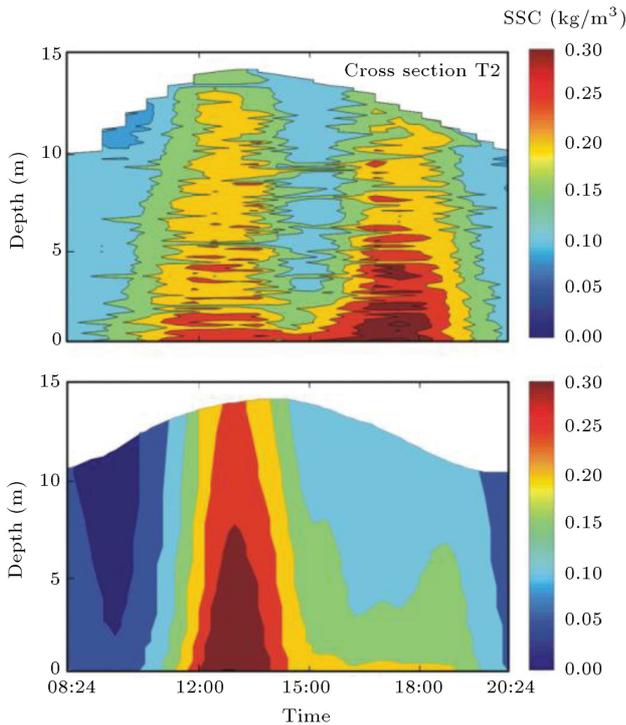


**Figure 5.** The evolution of the vertical profiles of SSC at middle point of cross sections T1 (the top graph) and T2 (the bottom graph).

high concentration of suspended sediment is achieved during the flood phase. During the ebb phase, however, an increase in SSC starts to shape at the bottom, which is terminated before its full development toward the surface.

To find out whether the evolution of the vertical profile of the SSC derived from field data follows the same rule at cross section T2, a similar graph for the period when the measured data was available was prepared to compare with that derived from the model. That is, from 08:24 to 20:24 on March 23rd, 2000 (Figure 6). Unlike the model results, formation of two successive high concentrations of suspended sediment during the flood and ebb phase can be observed regarding field data. It can also be seen that the two peaks are relatively close to each other (about a 4-hour interval between the two peaks) and that in the near bed region, the SSC remains high during the times between the two peaks, with values of about  $0.25 \text{ kg/m}^3$ . This indicates that the sediments suspended during the flood phase had insufficient time to settle onto the bed completely. Thus, during the returning ebb, the current caused a further increase in the concentration of suspended sediment, that is, the explanation for the peak of SSC, because of the ebb current being higher than the peak, due to the flood current.

Referring to the graph for the model, as discussed, it seems that the established model is incapable of reproducing the peak SSC during the ebb phase at this cross section. The model deficiency mentioned above



**Figure 6.** Evolution of the vertical profiles of SSC from the field data (the top graph) and the model results (the bottom graph) at the monitoring point at the middle of the cross section T2.

was observed in all monitoring points of cross section T2 (see Table 1).

From the results, it can be concluded that this incapability of the model might be the main reason for the deviations observed between the model and measured data at this particular cross section. The insufficient supply of sediment during the ebb condition is responsible for this behaviour of the model. One of the parameters responsible for this behaviour is the grain size distribution of sediment particles introduced to the model. As reported by Escobar (2007) [5], the prepared map of grain size distribution was based on a limited number of measurements, specifically on the tidal flat areas. He pointed out the existence of some localized discrepancies between values in the map and actual values in the field. He also mentioned that precise information regarding grain size distribution could improve the model results.

The other factor that might be responsible for the insufficient supply of sediment is the use of a constant settling velocity for the channels and the tidal flats. Distribution of different grain size in the area is necessitated using different settling velocities. Analysis of laboratory and field data has shown that the settling velocity of the flocs is related to the sediment concentration [13], the water depth [14], the flow velocity [15], and flocculation and biological activities [16]. Thus, the settling velocity for tidal channels cannot be the

same as that for tidal flat areas, because of flow depth and biological activity in the tidal flat. However, the use of variable settling velocities as a map is not yet incorporated into the Delft3D model.

The third factor to be considered for the observed model deficiency is the assignment of a constant erosion rate for the whole area, due to lack of available field data. The erosion rate is defined in the model via the Critical Bed Shear Stress for Erosion (CBSSE) and the constant erosion parameter (Eq. (2)):

$$E = M_{er}(\tau_b - \tau_{ce}) \text{ for } \tau_b > \tau_{ce}, \quad (2)$$

where  $\tau_b$  and  $\tau_{ce}$  are bed shear stress and critical surface erosion shear stress, respectively,  $M_{er}$  and is the mass erosion rate constant.

The difficulties inherent in measuring this parameter in the field have prevented the preparation of a map of the distribution of CBSSE for the area. Therefore, this parameter is defined to the model as a single value for the whole area under investigation.

A combination of the above mentioned factors are involved in the deficiency observed in the performance of the model, specifically, at cross section T2.

#### 4.2. Snapshots of suspended sediment concentrations in cross sections

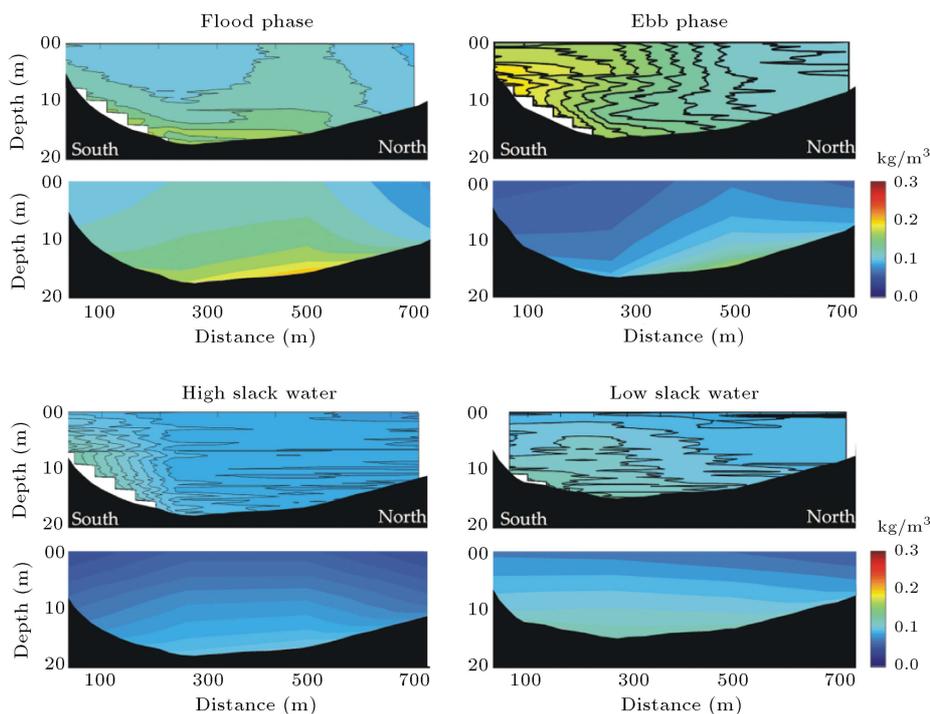
As another effort, the patterns of SSC distribution along cross sections T1 and T2 predicted by the model were prepared and compared with those derived from the measured data. That is, the snapshot of the distribution of SSC for cross sections T1 and T2 during different phases of a tidal cycle were prepared for both model results and field data.

Figures 7 and 8 show snapshots of times of flood, high slack water, ebb, and low slack water for cross sections T1 and T2, respectively. The top snapshot for each pair is from the field data and the one below is from the model results.

As expected, the snapshots in Figure 7 show good agreement between the modeled and measured SSC during different tidal phases at cross section T1. The concentration of suspended sediment is quite low in this cross section, and never exceeds  $0.2 \text{ kg/m}^3$  during a full tidal cycle.

According to Figure 8, however, the distribution of SSC derived from the model and measured data are not in good agreement for all tidal phases. It can be seen that during the flood phase, both the model and the field plots show the same pattern of distribution of SSC across the cross section. That is, the concentration, which is high in the bed to the middle layers of the southern bank region, decreases gradually towards the surface and the northern bank region. The SSC values are higher for the model plot.

At the following high slack water, the model plot shows high SSC at the deep region of the cross section

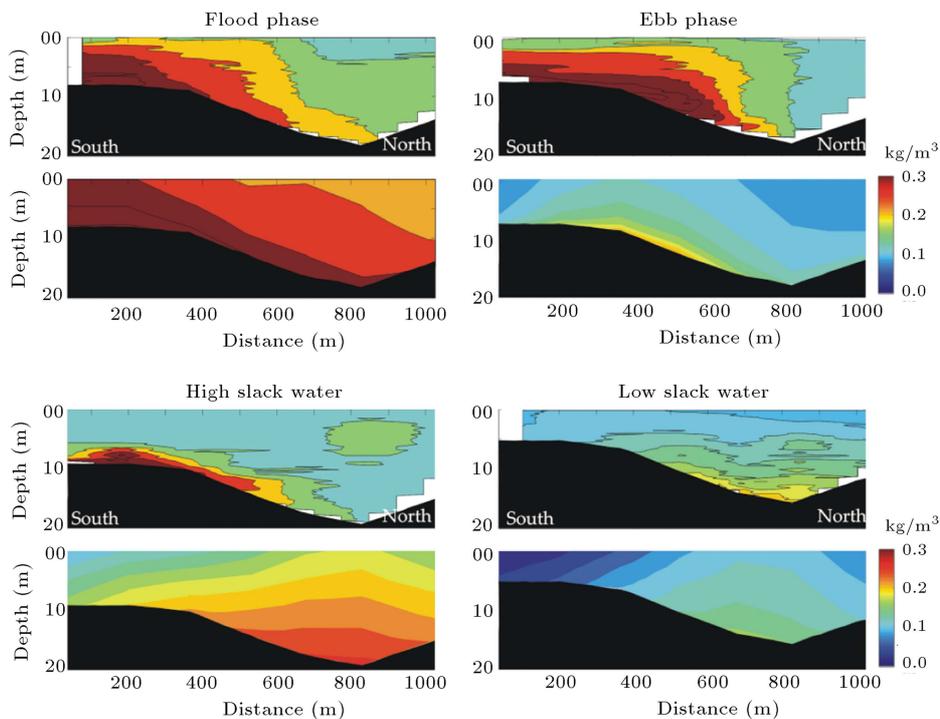


**Figure 7.** Snapshots of SSC distribution for flood, high slack water, ebb, and low slack water phase at cross-section T1. The top snapshots of each pair represent field data and the bottom ones show model results.

near the northern bank, but the field plot shows high SSC in the shallow region of the cross section near the southern bank. This plot also shows that the SSC decreases abruptly toward the surface and the northern

bank region, with the exception of an area of a high concentration in the middle of the cross section.

During the following ebb phase, the field plot shows the same pattern as that during the flood phase,



**Figure 8.** Snapshots of SSC distribution for flood, high slack water, ebb, and low slack water phase at cross-section T2. The top snapshots of each pair represent field data and the bottom ones show model results.

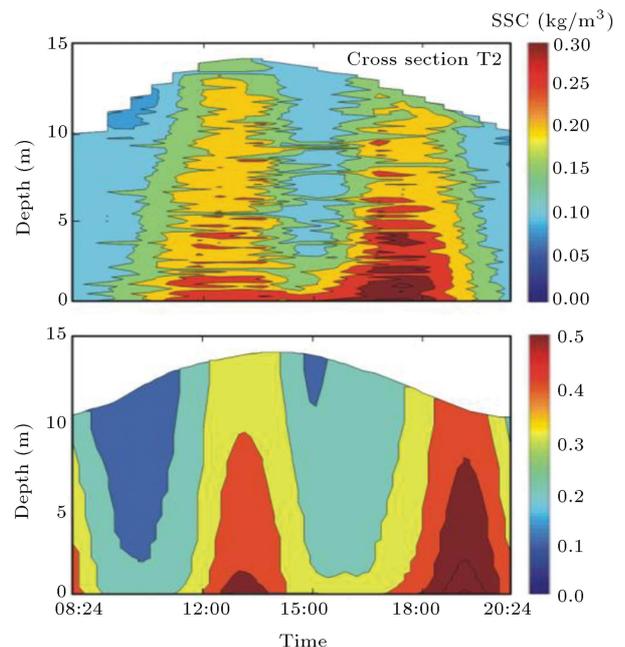
with high SSC in the southern bank, decreasing gradually toward the northern bank. In the model plot, it can be seen that the same process starts to develop in the near bed layer. The process however, failed to develop towards the surface, which was also observed previously in Figure 6. At low slack water, both model and field plots show the same pattern of SSC distribution, with the maximum SSC at the deepest position of the cross section decreasing gradually towards the surface. Referring to the model plots of this cross section for the four tidal phases, it is seen that the values of the SSC for the ebb phase and the low slack water are much lower in comparison with those for the flood phase and high slack water. The reason for the low values of SSC during the ebb phase of this cross section was discussed in detail in Section 4.1.

The concentration of suspended sediment at this cross section is relatively high, especially in the southern bank of the section, with the values exceeding up to  $0.35 \text{ kg/m}^3$  during ebb and flood phases.

#### 4.3. The effect of critical bed shear stress

It is mentioned in Section 4.1 that the reason for considering the constant erosion rate in the area was the lack of field data. It is, however, obvious that the Critical Bed Shear Stress for Erosion (CBSSE), thus, the erosion rate, is not the same everywhere in the area, specifically, not the same on the tidal flat and in the tidal channel. As a trial, a map has been prepared for the CBSSE, with the values for the tidal flat area being 50% less than those of the tidal channel. That is, to allow more sediment to be suspended and transported by the ebb current from the tidal flat area. The map is introduced to the model instead of a plain value of erosion rate. Using this map, the model was executed. The result, which is presented in Figure 9, shows successive peaks of SSC due to the ebb current as well as the flood current. It can also be seen that the peak of SSC during the ebb phase is higher than that during the flood phase, which is the case for the field data. It should, however, be mentioned that the usage of this map of CBSSE has caused considerable increase in the SSC values for the whole period of tidal condition. That is, the concentration of suspended sediment is elevated to about  $0.5 \text{ kg/m}^3$ . It should be emphasized that the map of CBSSE was prepared by the author, executing several trials. It is not made on the basis of field measurements.

The output of this model is also used to prepare snapshot plots of the cross section with the revised values for CBSSE (Figure 10). Referring to this figure, successive performance of the model was achieved during the ebb phase. That is, the peak of SSC influenced by the ebb current is reproduced by the model. However, as seen, overprediction is imposed



**Figure 9.** Evolution of the vertical profiles of SSC from the field data (the top graph) and from the model results using low value of critical bed shear stress for erosion for the tidal flat and high value for tidal channel (the bottom graph).

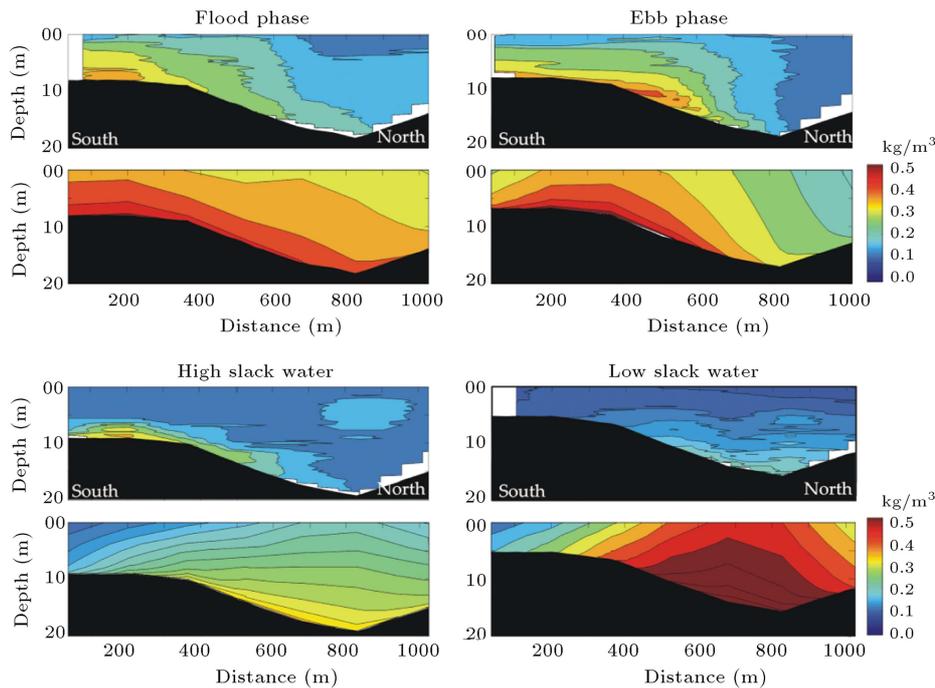
onto SSC values computed by the model all over the cross section and for all tidal stages. This was also the case in Figure 9.

## 5. Conclusion

To evaluate the ability of the Delft3D model to predict Suspended Sediment Concentration (SSC) of the Piep tidal channel system, modeled data were compared with field data using two different experiments, including evolution of the vertical profiles of SSC in a monitoring point at the middle of the cross sections, and the snapshots of SSC in the cross sections.

In comparative analyses of the SSC profiles derived from the model with those derived from the field, some dissimilarities were observed relating to the ebb current and low slack water of cross sections T2. That is, the model could not simulate the peak SSC during the ebb current at this cross section.

The insufficient supply of sediment from the tidal flat area on the eastern side of this cross section was found to be responsible for this behaviour of the model. In other words, the modelled tidal flat areas do not supply sufficient sediment during the ebb current. Several parameters and/or factors have been mentioned as being responsible for this insufficient supply of sediment, including the grain size map introduced to the model, the usage of one plain settling velocity for the whole area, and the usage of a constant value of erosion rate for the area.



**Figure 10.** Snapshots of SSC distribution for flood, high slack water, ebb, and low slack water phase at cross-section T2. The top snapshots of each pair represent field data and the bottom ones show model results (employed revised CBSSE).

According to Wiberg (2012) [17], sediment suspension in Delft3D, for a given bed shear stress, is controlled primarily by settling velocity and an erosion rate parameter. Critical shear stress must also be specified, but it is not allowed to vary with depth or mass eroded. The paper also mentioned that SSC in the model calculations is more sensitive to the erosion rate parameter than to settling velocity.

It is also reported by Gourgue et al. (2013) [18] that the settling velocity of suspended sediments is influenced by flocculation. The important factors governing this process include the SSC itself, turbulence, shear stress, salinity, biological activity and some physicochemical properties. It, therefore, cannot be considered the same for different spatial conditions.

It should also be emphasized that the hydrodynamic of the model itself can be responsible for some deficiency observed in the SSC predicted by the model. That is, even though the hydrodynamic of the model was verified by as good as 80%, it can still cause part of the shortcomings observed in the model results.

The input of different values of the Critical Bed Shear Stress for Erosion (CBSSE) for the tidal flat areas and the tidal channel eastward of cross section T2 did improve the model results.

The necessity for the production of a CBSSE map on the basis of field data and model simulations is, therefore, suggested for improvement of the model results.

In short, it can be concluded that the developed

model is sufficiently accurate to provide values of the suspended material concentration comparable with those of field data to a certain extent. It, however, should be emphasized that, in order to achieve reliable results, precise field measurements all over the area, with the aim of estimating critical bed shear stress for erosion, need to be carried out.

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### Biography

**Maryam Rahbani** was born in Shiraz, Iran. She obtained her BS degree in Mechanical Engineering from Sharif University of Technology, Tehran, Iran, in 1991, and her MS degree in Physical Oceanography from the University of Chamran, Iran, in 1999 (the intrusion of Anzali wetlands into the Caspian Sea, using numerical modelling). She moved to Germany in 2004 to pursue her PhD degree (thesis Entitled “Numerical modelling of the coastal processes in Dithmarschen Bight, incorporating field data”) using the Delft3D model, a well-known software in oceanographic sciences. She is currently Assistant Professor at the University of Hormozgan, Iran.